College Station, TX



City Hall 1101 Texas Ave College Station, TX 77840

Legislation Details (With Text)

File #: 18-0540 Version: 1 Name: Loveless Tract CPA

Type: Comprehensive Plan Status: Agenda Ready

File created: 8/9/2018 In control: City Council Regular

On agenda: 8/23/2018 Final action:

Title: Public Hearing, presentation, possible action, and discussion regarding an ordinance amending the

Comprehensive Plan - Future Land Use & Character Map from Suburban Commercial to Urban for approximately 11.1 acres generally located in the City on the west side of Turkey Creek Road,

approximately 500-feet south of HSC Parkway.

Sponsors: Alaina Helton

Indexes:

Code sections:

Attachments: Loveless background

Vicinity & Aerial Map

Future Land Use and Character Map

Ordinance

Date Ver. Action By Action Result

Public Hearing, presentation, possible action, and discussion regarding an ordinance amending the Comprehensive Plan - Future Land Use & Character Map from Suburban Commercial to Urban for approximately 11.1 acres generally located in the City on the west side of Turkey Creek Road, approximately 500-feet south of HSC Parkway.

Relationship to Strategic Goals:

- Good Governance
- Core Services and Infrastructure
- Neighborhood Integrity
- Diverse Growing Economy

Recommendation(s): The Planning and Zoning Commission considered this item on August 2, 2018 and voted 7-0 to recommend approval.

Summary: The applicant is requesting the proposed future land use amendment from Suburban Commercial to Urban. This development is intended to serve the nearby BioCorridor Planned Development District and to provide supplemental uses that add to the efforts made in that area. The subject property and properties to the south are designated as Suburban Commercial. Properties to the north, east and west are designated Business Park on the Comprehensive Plan Future Land Use and Character Map.

REVIEW CRITERIA

1. Changed or changing conditions in the subject area or the City: The City of College

Station's Comprehensive Plan was adopted in 2009. In 2010, through the coordination of the Research Valley Partnership, the Research Valley BioCorridor Concept Master Plan was developed, consisting of a conceptual area of approximately 3,500 acres, spanning the jurisdictions of College Station and Bryan and including property owned by the Texas A&M University System. In 2012, approximately 200 acres west of Turkey Creek Road were zoned BioCorridor Planned Development District (PDD) to encourage research and development, manufacturing, and warehousing for biomedical and other emerging technology industries in the area. The area surrounding this PDD is anticipated to serve as a secondary or supplementary region that would provide supportive land uses enhancing the development occurring within the BioCorridor District. The BioCorridor Concept Master Plan envisioned a mix of uses in the subject area; thus prompting the request to amend the Future Land Use map to Urban.

Development pressure in this area has increased since the conception of the BioCorridor Master Plan, and the City of College Station has made significant agreements and infrastructure investments in the nearby area (e.g., roadway and electric improvements and changes in sewer certificated areas) to support the success of business development in the region. In addition, three properties in the vicinity of the BioCorridor PDD have had their land use designations changed to Urban in the past couple of years to accommodate the development of supporting uses to the BioCorridor and in anticipation of land use demands related to the RELLIS campus in Bryan.

2. Compatibility with the existing uses, development patterns, and character of the immediate area concerned, the general area, and the City as a whole: Currently, the Comprehensive Plan Future Land Use and Character Map designates the area west of Turkey Creek Road and north of Raymond Stotzer Parkway to be approximately 37 acres of Suburban Commercial, 11 acres of Urban and 198 acre of Business Park, the majority of which is zoned BioCorridor Planned Development District.

The Comprehensive Plan Future Land Use and Character Map designates the area to the east of Turkey Creek Road and west of Harvey Mitchell Parkway to be approximately 23 acres of Suburban Commercial (property already zoned for commercial uses along Harvey Mitchell), 17 acres of Urban, 6 acres of General Commercial, 39 acres to be Texas A&M University (property owned by Texas A&M), 29 acres of Public/Institutional (the City's cemetery), and the largest designation to be 54 acres of Business Park.

The proposal is to convert approximately 11 acres from Suburban Commercial to Urban on the west side of Turkey Creek Road, approximately 500-feet south of HSC Parkway. The proposed amendment would allow for a very intense level of residential development activities being townhouse, duplexes, and high-density apartments.

The existing adjacent land uses to the east and south are large lot single-family and unimproved rural tracts varying in size from four to seven acres. The property to the north and west is currently undeveloped and is located within the BioCorridor Planned Development District. These existing rural developments are less dense than the Business Park designation prescribes. An Urban development will provide a different character than these existing properties currently provide.

Land located north, east and west of the subject property is anticipated for future Business Park, and property to the south, Suburban Commercial land uses. The proposed amendment, while not

consistent with the planned future development pattern in the area, is compatible as a supporting land use for the Business Park uses that have begun to develop in the BioCorridor PDD.

- 3. Impact on environmentally sensitive and natural areas: A small pond is located on the property, but this area is not recognized as environmentally sensitive, nor is there any FEMA regulated floodplain onsite.
- 4. Impacts on infrastructure, including water, wastewater, drainage, and the transportation network: The Urban designation allows for future development of high-density residential uses such as townhomes, duplexes and multi-family. Water service is provided to properties in this area by the City of College Station and Wellborn Special Utility District (SUD), but additional infrastructure improvements may be needed with further site development to meet minimum fire flow requirements. With future development of the site there will be an offsite extension of waterline required to connect to the existing 18-inch water main on the south side of HSC Pkwy. The extension will be a 12-inch water line constructed with the extension of Atlas Pear Drive that will serve the tract for the portion of the property served by the City of College Station.

Sanitary sewer for the property will be provided by the City of Bryan, and extensions to provide service to the property may be needed with further site development. Sanitary sewer lines will be required to be extended to serve this property at the time of site development.

Drainage is generally to the south within the Whites Creek Drainage Basin. Currently, there is no FEMA regulated flood plain on site, however, detention will be required with site development. Drainage and other public infrastructure required with the site shall be designed and constructed in accordance with the B/CS Unified Design Guidelines.

The subject tract has frontage to Turkey Creek Road which is designated as a future Major Collector on the Thoroughfare Plan. Turkey Creek Road is currently constructed as two-lane rural section roadway and is under TxDOT jurisdiction as FM 2513. The applicant also anticipates driveway access to Atlas Pear Drive which would be extended with development south from HSC Parkway and be designed as Minor Collector.

5. Consistency with the goals and strategies set forth in the Comprehensive Plan: The existing Future Land Use and Character designation on the property is Suburban Commercial, which is defined as a land use designation "generally for concentrations of commercial activities that cater primarily to nearby residents versus the larger community or region. Generally, these areas tend to be small in size and located adjacent to major roads (arterials and collectors). Design of these structures is compatible in size, roof type and pitch, architecture, and lot coverage with the surrounding single-family residential uses".

While the land use designation is still valid and could support the growth of a research and development and manufacturing district, none of the surrounding properties are zoned or designated for single-family development, except for the adjacent properties that are zoned Rural, which are also designated as Suburban Commercial and Business Park on the Future Land Use and Character Map, and are anticipated to develop as such in the future. The proposed Urban land use also encourages growth, by providing workforce housing needed to support commercial uses in the area.

Urban is described as:

File #: 18-0540, Version: 1

Generally for areas that should have a very intense level of development activities. These areas will tend to consist of townhomes, duplexes, and high-density apartments. General commercial and office uses, business parks, and vertical mixed-use may also be permitted within growth and redevelopment areas.

The Urban land use designation, when not in a Growth Area, is typically characterized by more dense residential developments in the form of duplexes, townhomes or apartments.

The subject tract has frontage to Turkey Creek Road which is designated as a future Major Collector on the Thoroughfare Plan. Turkey Creek Road is currently constructed as two-lane rural section roadway. The applicant anticipates driveway access to Atlas Pear Drive which would be extended with development south from HSC Parkway and be designed as Minor Collector.

The proposal is consistent with the goals and strategies of the Comprehensive Plan. The goal for College Station's Future Land Use and Character is to create a community with strong, unique neighborhoods, protected rural areas, special districts, distinct corridors, and a protected and enhanced natural environment. The subject property and surrounding area are identified in the Comprehensive Plan Concept Map as one of the City's distinct districts-the Presidential Corridor Gateway District-which seeks to attract business (research and development, office, and light industrial) and preserve and build upon the existing assets in the area enhancing this important gateway into the city. The proposed amendment from Suburban Commercial to Urban on just over 11 acres does not limit the general goals of the Comprehensive Plan. The Urban land use encourages growth, by providing workforce housing needed to support business uses in the area.

Budget & Financial Summary: N/A

Legal Review: Yes

Attachments:

- 1. Background Information
- 2. Vicinity & Aerial Map
- 3. Comprehensive Plan Future Land Use and Character Map
- 4 Ordinance

NOTIFICATIONS

Advertised Commission Hearing Date: August 2, 2018
Advertised Council Hearing Dates: August 23, 2018

The following neighborhood organizations that are registered with the City of College Station's Neighborhood Services have received a courtesy letter of notification of this public hearing:

None

Contacts in support: None Contacts in opposition: None

Inquiry contacts:

One at the time of staff report.

ADJACENT LAND USES

		1			
Direction	Comprehensive Plan	Zoning	Land Use		
North	Business Park	Planned Development District (BioCorridor)	Rural		
South	Suburban Commercial	Rural	Rural		
East (across Turkey Creek Road)	Business Park	Rural	Single-Family Residential		
West	Business Park	Planned Development District (BioCorridor)	Rural		

DEVELOPMENT HISTORY

Annexation: June 21, 1995

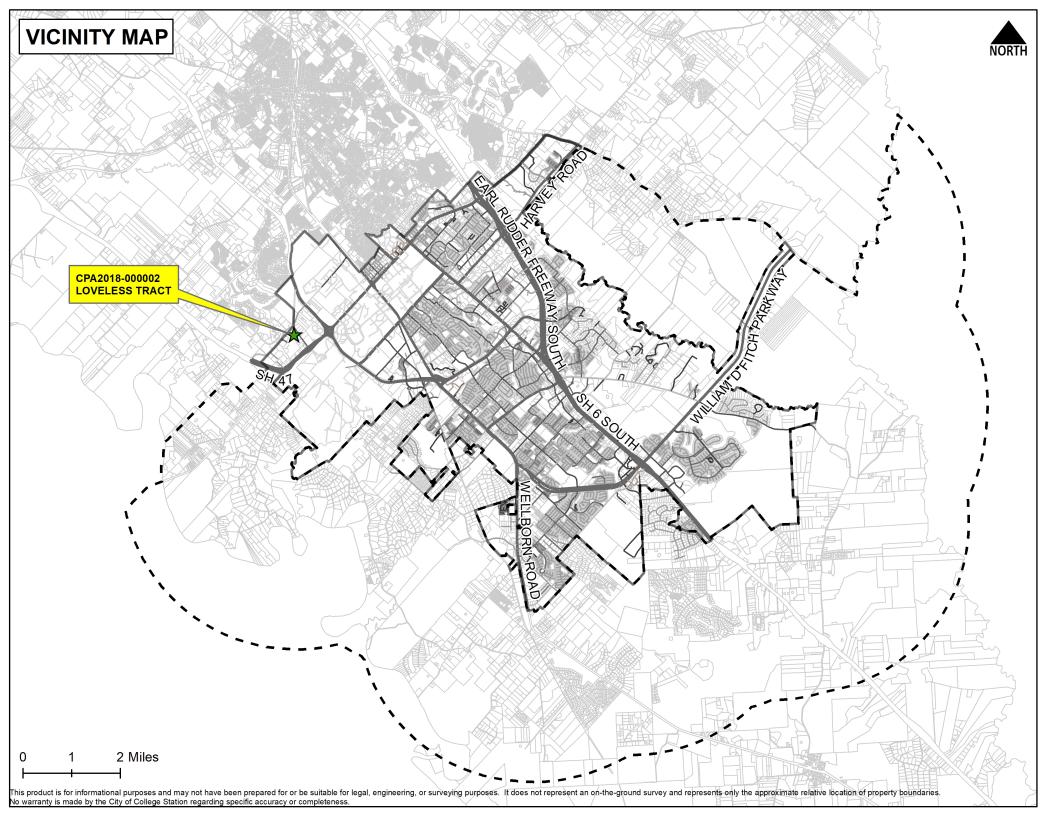
Zoning: A-O Agricultural Open upon annexation

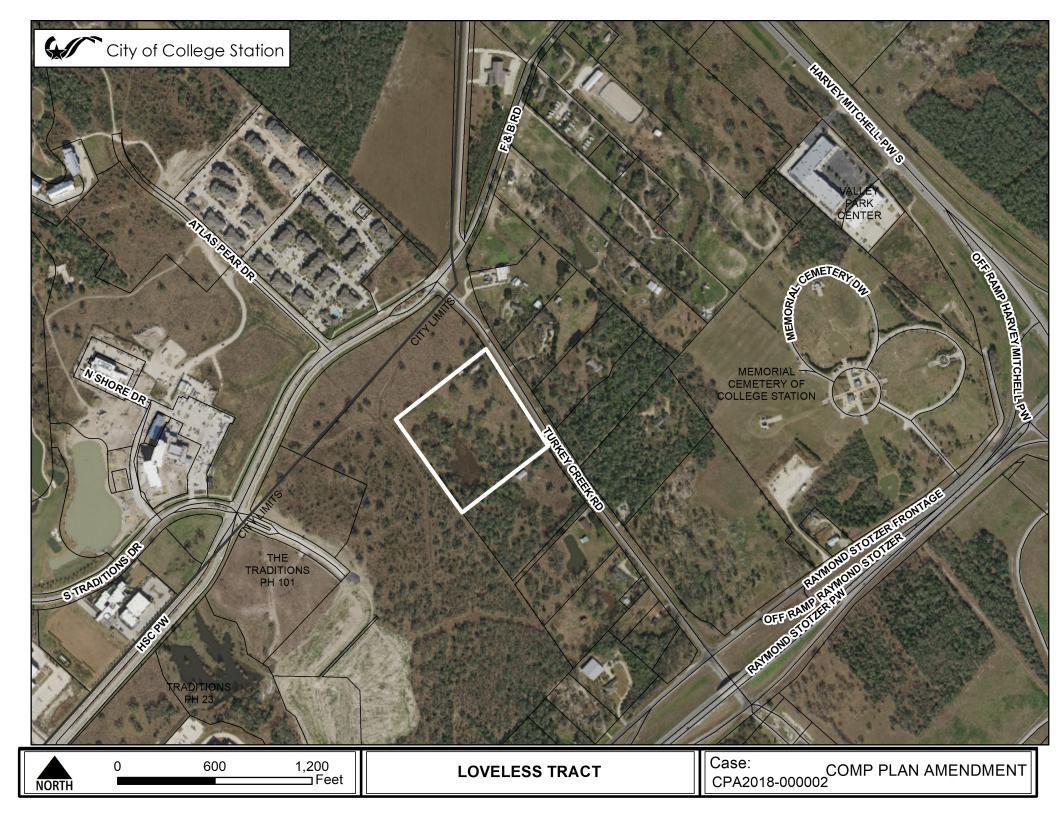
A-O Agricultural Open renamed to R Rural in 2013

Final Plat: Property is not platted

Site development: Property is generally undeveloped. It is the site of a former horse

stable.





EXISTINGFuture Land Use

Texas A&M University Bustness Park Suburban Commercial Urban **Business Park**

PROPOSEDFuture Land Use



	ORDINANCE NO
COLLEGE ST PLAN – FUT TO URBAN F ON THE WES OF HSC PA	NCE OF THE CITY OF COLLEGE STATION, TEXAS, AMENDING THE FATION COMPREHENSIVE PLAN BY AMENDING THE COMPREHENSIVE URE LAND USE & CHARACTER MAP FROM SUBURBAN COMMERCIAL FOR APPROXIMATLEY 11.1 ACRES GENERALLY LOCATED IN THE CITY ST SIDE OF TURKEY CREEK ROAD, APPROXIMATELY 500-FEET SOUTH ARKWAY; PROVIDING A SEVERABILITY CLAUSE; PROVIDING AN DATE; AND CONTAINING OTHER PROVISIONS RELATED THERETO.
BE IT ORDATEXAS:	AINED BY THE CITY COUNCIL OF THE CITY OF COLLEGE STATION,
PART 1:	That the "Comprehensive Plan of the City of College Station" is hereby amended by adding new Subsections C.2.ii of Exhibit "A" thereto as set out in Exhibit "A" attached hereto and made a part hereof; and by amending the "Comprehensive Plan Future Land Use and Character Map," as set out in Exhibit "B" attached hereto and made a part hereof for the identified area.
PART 2:	That if any provisions of any section of this Ordinance shall be held to be void or unconstitutional, such holding shall in no way affect the validity of the remaining provisions or sections of this Ordinance, which shall remain in full force and effect.
PART 3:	That this Ordinance shall take effect immediately from and after its passage.

APPROVED:

Mayor

PASSED, ADOPTED and APPROVED this 23rd day of August, 2018.

ATTEST:

City Secretary

APPROVED:

City Attorney

EXHIBIT A

That ordinance no. 3186 adopting the "Comprehensive Plan of the City of College Station" as amended, is hereby amended by adding new subsections C.2.ii. to Exhibit "A" of said plan for Exhibit "A" to read in its entirety as follows:

"EXHIBIT 'A'

A. Comprehensive Plan

The College Station Comprehensive Plan (Ordinance 3186) is hereby adopted and consists of the following:

- 1. Introduction;
- 2. Community Character;
- 3. Neighborhood Integrity;
- 4. Economic Development;
- 5. Parks, Greenways & the Arts;
- 6. Transportation;
- 7. Municipal Services & Community Facilities;
- 8. Growth Management and Capacity; and
- 9. Implementation and Administration.

B. Master Plans

The following Master Plans are hereby adopted and made a part of the College Station Comprehensive Plan:

- 1. The Northgate Redevelopment Plan dated November 1996;
- 2. The Revised Wolf Pen Creek Master Plan dated 1998;
- 3. Northgate Redevelopment Implementation Plan dated July 2003;
- 4. East College Station Transportation Study dated May 2005;
- 5. Bicycle, Pedestrian, and Greenways Master Plan dated January 2010;
- 6. Central College Station Neighborhood Plan dated June 2010;
- 7. Eastgate Neighborhood Plan dated June 2011;
- 8. Parks, Recreation and Open Spaces Master Plan dated July 2011;
- 9. Southside Area Neighborhood Plan dated August 2012;
- 10. Medical District Master Plan dated October 2012;
- 11. Wellborn Community Plan dated April 2013;
- 12. Economic Development Master Plan dated September 2013;
- 13. South Knoll Area Neighborhood Plan dated September 2013;
- 14. The Water System Master Plan dated April 2017; and
- 15. The Wastewater System Master Plan dated April 2017.

C. Miscellaneous Amendments

The following miscellaneous amendments to the College Station Comprehensive Plan are as follows:

1. Text Amendments:

- a. Chapter 2 "Community Character," "Growth Areas" by amending the text regarding Growth Area IV and Growth Area V Ordinance 3376, dated October 2011.
- b. Chapter 6 "Transportation" by amending the text regarding Complete Streets, Context Sensitive Solutions, Minimum Length and Additional Right-of-Way for Turn Lanes at Intersections, and Right-of-Way for Utilities – Ordinance 3729, dated December 10, 2015.
- c. Chapter 2 "Community Character," Chapter 3 "Neighborhood Integrity," Chapter 4 "Economic Development," Chapter 5 "Parks, Greenways, and the Arts," and Chapter 7 "Municipal Services and Community Facilities" by amending the text based on the recommendation of the Comprehensive Plan Five-Year Evaluation & Appraisal Report Ordinance 3730 dated December 10, 2015.
- d. Chapter 8 "Growth Management & Capacity" by amending the text based on recommendations from the Annexation Task Force – Ordinance 3766, dated April 28, 2016.
- e. Chapter 1, Section 10; Chapter 5, Section 1; Chapter 5, Section 4; Chapter 8, Section 17; and Chapter 9, Section 3; by amending the text Ordinance 3951, dated October 12, 2017.

2. Future Land Use and Character Map Amendment:

- a. 301 Southwest Parkway Ordinance 3255, dated July 2010.
- b. Richards Subdivision Ordinance 3376, dated October 2011.
- c. Earl Rudder Freeway at University Oaks Ordinance 3465, dated November 19, 2012
- d. 1600 University Drive East Ordinance 3535, dated November 14, 2013.
- e. 2560 Earl Rudder Freeway S. Ordinance 3541, dated December 12, 2013.
- f. 13913 FM 2154. Ordinance 3546, dated January 9, 2014.
- g. 2021 Harvey Mitchell Parkway Ordinance 3549, dated January 23, 2014.
- h. 1201 Norton Lane Ordinance 3555, dated February 27, 2014.
- i. 3715 Rock Prairie Road West Ordinance 3596, dated August 25, 2014.
- j. 4201 Rock Prairie Road Ordinance 3670, dated July 9, 2015.
- k. The approximately 40 acres of land generally located east of FM 2154 (aka Wellborn Road), south of the Southern Trace Subdivision, west of State Highway 40 (aka William D. Fitch Parkway), and north of Westminster Subdivision Ordinance 3731, dated December 10, 2015.
- The approximately 120 acres of land generally located south of Barron Cut-Off Road, west of WS Phillips Parkway, north of the Castlegate II Subdivision, and east of the Wellborn Community – Ordinance 3732, dated December 10, 2015.
- m. The approximately 900 acres of land generally located south of Greens Prairie Road West, east of the Sweetwater Subdivision, and north of Arrington Road – Ordinance 3733, dated December 10, 2015.
- n. The approximately 17.788 acres of land generally located at the corner of Turkey Creek Road and Raymond Stotzer Parkway frontage road.—Ordinance 3752, dated March 10, 2016.

- The approximately 9 acres of land generally located north of the Crossroad Woods Subdivision near the intersection of Wellborn Road (FM 2154) and Greens Prairie Trail
 Ordinance 3779, dated June 9, 2016
- p. The approximately 16 acres located at 8607 Rock Prairie Road, generally located at the north of Rock Prairie Road and west of William D. Fitch Parkway – Ordinance 3794, dated August 11, 2016.
- q. The approximately 14.25 acres of land located at 2501 Earl Rudder Freeway South, generally located north of North Forest Parkway and south of Raintree Drive, along the east side of Earl Rudder Freeway South Ordinance 3799, dated August 25, 2016.
- r. The approximately 7 acres of land located along the south side of State Highway 30, south of Veterans Memorial Park Ordinance 3828, dated October 27, 2016.
- s. The approximately 58 acres of land generally located along the east side of State Highway 6 South, north of W.D. Fitch parkway and south of the future Pebble Creek Parkway extension –Ordinance 3830, dated October 27, 2016.
- t. The approximately 2 acres of land generally located on Corsair Circle north of Pavilion Avenue Ordinance 3846, dated December 8, 2016.
- The approximately 18 acres of land generally located at the southeast corner of Sebesta Road and Earl Rudder Freeway frontage road – Ordinance 3848, dated December 8, 2016.
- v. The approximately 6 acres of land being situated in the Pooh's Park Subdivision, Block 1, Lots 6-14 recorded in Volume 314, Page 618 of the deed records of Brazos County, Texas, located at 204, 206, 208, 210, 212, 214, 216, 218, and 220 Holleman Drive east, more generally southwest of the intersection of Holleman Drive East and Lassie Lane by Ordinance 3850, dated January 12, 2017.
- w. The approximately 6.3 acres of land generally located northeast intersection of Copperfield Parkway and Crescent Pointe Parkway - Ordinance 3859, dated February 9, 2017.
- x. The approximately 11.3 acres of land generally located at the southeast intersection of Earl Rudder Freeway South and Emerald Parkway Ordinance 3875, dated April 27, 2017.
- y. The approximately 232 acres of land generally located south of Rock Prairie Road Ordinance 3877, dated April 27, 2017.
- z. The approximately 5.96 acres of land generally located in the northeast corner of Associates Avenue and Harvey Road intersection. – Ordinance 3879, dated April 27, 2017.
- aa. The approximately 4.74 acres of land generally located at the corner of Harvey Mitchell Parkway South and Raymond Stotzer Parkway Ordinance 3882, dated May 11, 2017.
- bb. The approximately 3.2 acres of land generally located east of Copperfield Parkway and south of Harvey Road. Ordinance 3884, dated May 11, 2017.
- cc. The approximately 3.34 acres of land generally located north of Raintree Drive along Earl Rudder Freeway South Ordinance 3901, dated July 27, 2017.

- dd. The approximately 12.5 acres generally located west of Earl Rudder Freeway South along Harvey Mitchell Parkway Ordinance 3903, dated July 27, 2017.
- ee. Approximately 100 acres located along the east side of State Highway 6, south of The Business Park at College Station and north of the Texas World Speedway property-Ordinance 3906, dated July 27, 2017.
- ff. Approximately 1.7 acres of land being generally located at 12055 FM 2154 Ordinance 3921, dated August 24, 2017.
- gg. Approximately 11 acres of land being generally located at 8822 Burgess Lane and near the east intersection of State Highway 47 and Raymond Stotzer Parkway Ordinance 3965, dated December 14, 2017.
- hh. Approximately 35 acres of land being generally located at the west intersection of William D Fitch Parkway and Rock Prairie Road Ordinance 3989, dated January 25, 2018.
- Approximately 11.1 acres of land being generally located on the west side of Turkey Creek Road, approximately 500 feet south of HSC Parkway- by this Ordinance, dated August 23, 2018.

3. Concept Map Amendment:

- a. Growth Area IV Ordinance 3376, dated October 2011.
- b. Growth Area V Ordinance 3376, dated October 2011.

4. Thoroughfare Map Amendment:

- a. Raintree Drive Ordinance 3375, dated October 2011.
- b. Birkdale Drive Ordinance 3375, dated October 2011.
- c. Corsair Circle Ordinance 3375, dated October 2011.
- d. Deacon Drive Ordinance 3375, dated October 2011.
- e. Dartmouth Drive Ordinance 3375, dated October 2011.
- f. Farm to Market 60 Ordinance 3375, dated October 2011.
- g. Southwest Parkway Ordinance 3375, dated October 2011.
- h. Cain Road extension Ordinance 3639, dated February 26, 2015.
- i. Update to Chapter 6 Maps- Ordinance 3729, dated December 10, 2015.
- j. South College Station Mobility Study Ordinance 3827, dated October 27, 2016.
- k. Pavilion Avenue extension --Ordinance 3885, dated May 11, 2017.
- 1. Future Minor Collector realigned to Harpers Ferry Road along Arrington Road Ordinance 3949, dated October 12, 2017.
- m. 2017 Thoroughfare Plan update to Map 6.6 Ordinance 3962, dated November 20, 2017.

5. Bicycle, Pedestrian and Greenways Master Plan Amendment:

- a. Cain Road extension Ordinance 3639, dated February 26, 2015
- b. Update to Maps 5.4 and 5.5-- Ordinance 3729, dated December 10, 2015.
- c. South College Station Mobility Study Ordinance 3827, dated October 27, 2016.
- d. Update to Maps 5.4 and 5.5 Ordinance 3877, dated April 27, 2017.
- e. 2017 Thoroughfare Plan update to Maps 5.4 and 5.5 Ordinance 3962, dated November 20, 2017.

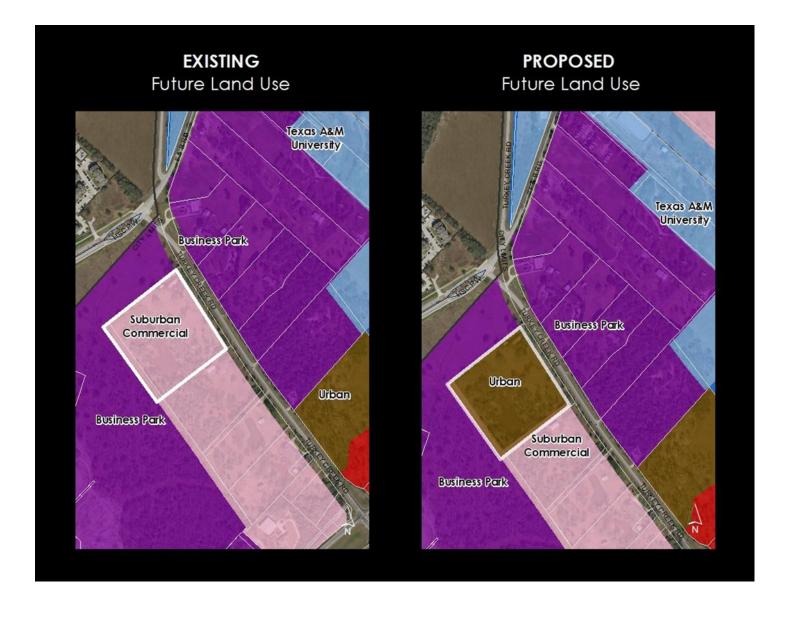
- f. 2018 Bicycle, Pedestrian, and Greenways Master Plan Update to Maps 5.4, 5.5, 7.1, 7.2, 7.3, and 7.4 –Ordinance 4009, dated May 14, 2018.
- 6. Parks and Recreation Master Plan Amendment:
 - a. Update to Map B/College Station Neighborhood Park Zone, to Map C/College Station Community Park Zone Map and to the text of Section 2 "Current Conditions and Trends," subsection "Park Land," paragraph five relating to said maps-Ordinance 3951, dated October 12, 2017.
- 7. Community and Neighborhood Plan Amendment:
 - a. Eastgate Neighborhood Plan, Chapter 1 Community Character, "Site Development Area 1 Lincoln Avenue" Ordinance 3956, dated November 9, 2017.

D. General

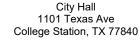
- 1. Conflict. All parts of the College Station Comprehensive Plan and any amendments thereto shall be harmonized where possible to give effect to all. Only in the event of an irreconcilable conflict shall the later adopted ordinance prevail and then only to the extent necessary to avoid such conflict. Ordinances adopted at the same city council meeting without reference to another such ordinance shall be harmonized, if possible, so that effect may be given to each.
- 2. Purpose. The Comprehensive Plan is to be used as a guide for growth and development for the entire City and its extra-territorial jurisdiction ("ETJ"). The College Station Comprehensive Plan depicts generalized locations of proposed future land-uses, including thoroughfares, bikeways, pedestrian ways, parks, greenways, and waterlines that are subject to modification by the City to fit local conditions and budget constraints.
- 3. General nature of Future Land Use and Character. The College Station Comprehensive Plan, in particular the Future Land Use and Character Map found in A.3 above and any adopted amendments thereto, shall not be nor considered a zoning map, shall not constitute zoning regulations or establish zoning boundaries and shall not be site or parcel specific but shall be used to illustrate generalized locations.
- 4. General nature of College Station Comprehensive Plan. The College Station Comprehensive Plan, including the Thoroughfare Plan, Bicycle, Pedestrian, and Greenways Master Plan, Central College Station Neighborhood Plan, Water System Master Plan and any additions, amendments, master plans and subcategories thereto depict same in generalized terms including future locations; and are subject to modifications by the City to fit local conditions, budget constraints, cost participation, and right-of-way availability that warrant further refinement as development occurs. Linear routes such as bikeways, greenways, thoroughfares, pedestrian ways, waterlines and sewer lines that are a part of the College Station Comprehensive Plan may be relocated by the City 1,000 feet from the locations shown in the Plan without being considered an amendment thereto.
- 5. Reference. The term College Station Comprehensive Plan includes all of the above in its entirety as if presented in full herein, and as same may from time to time be amended."

EXHIBIT B

That the "Comprehensive Plan of the City of College Station" is hereby amended by amending a portion of the map titled "Map 2.2-Future Land Use & Character" of Chapter 2 —Community Character" from Suburban Commercial to Urban shown as follows:









Legislation Details (With Text)

File #: 18-0542 Version: 2 Name: Loveless Tract REZ

Type: Rezoning **Status:** Agenda Ready

File created: 8/9/2018 In control: City Council Regular

On agenda: 8/23/2018 Final action:

Title: Public Hearing, presentation, possible action, and discussion regarding an ordinance amending

Appendix "A", "Unified Development Ordinance," Section 4.2, "Official Zoning Map," of the Code of Ordinances of the City of College Station, Texas by changing the zoning district boundaries from Rural

to Multi-Family on approximately 11.1 acres located on the west side of Turkey Creek Road,

approximately 500-feet south of HSC Parkway.

Sponsors: Alaina Helton

Indexes:

Code sections:

Attachments: Loveless background

Vicinity, Aerial & Small Area Map

Rezoning Map

TIA Loveless Rezoning

Ordinance

Date Ver. Action By Action Result

Public Hearing, presentation, possible action, and discussion regarding an ordinance amending Appendix "A", "Unified Development Ordinance," Section 4.2, "Official Zoning Map," of the Code of Ordinances of the City of College Station, Texas by changing the zoning district boundaries from Rural to Multi-Family on approximately 11.1 acres located on the west side of Turkey Creek Road, approximately 500-feet south of HSC Parkway.

Relationship to Strategic Goals:

- Good Governance
- Financially Sustainable City
- Core Services and Infrastructure
- Diverse Growing Economy

Recommendation: The Planning and Zoning Commission considered this item on August 2, 2018 and voted 5-2 to recommend approval. Dissenting votes were based on the lack of adequate road infrastructure to support the rezoning at this time.

Summary: This request is to rezone approximately 11.1 acres from R Rural to MF Multi-Family. This rezoning is in conjunction with a request to amend the Comprehensive Plan Future Land Use and Character Map by changing the designation of this property from Suburban Commercial to Urban.

NOTIFICATIONS

Advertised Commission Hearing Date: August 2, 2018
Advertised Council Hearing Dates: August 23, 2018

The following neighborhood organizations that are registered with the City of College Station's Neighborhood Services have received a courtesy letter of notification of this public hearing:

None

Contacts in support: None Contacts in opposition: None

Inquiry contacts: One at the time of staff report.

ADJACENT LAND USES

Direction	Comprehensive Plan	Zoning	Land Use		
North	Business Park	Planned Development District (BioCorridor)	Rural		
South	Suburban Commercial	Rural	Rural		
East (across Turkey Creek Road)	Business Park	Rural	Single-Family Residential		
West	Business Park	Planned Development District (BioCorridor)	Rural		

DEVELOPMENT HISTORY

Annexation: June 21, 1995

Zoning: A-O Agricultural Open upon annexation

A-O Agricultural Open renamed to R Rural in 2013

Final Plat: Property is not platted

Site development: Property is generally undeveloped. It is the site of a former horse

stable.

File #: 18-0542, Version: 2

REZONING REVIEW CRITERIA

1. Whether the proposal is consistent with the Comprehensive Plan: This request is in conjunction with a request to amend the Comprehensive Plan Future Land Use and Character Map by changing the designation of this property from Suburban Commercial to Urban.

The City of College Station's Comprehensive Plan was adopted in 2009. In 2010, through the coordination of the Research Valley Partnership, the Research Valley BioCorridor Concept Master Plan was developed, consisting of a conceptual area of approximately 3,500 acres, spanning the jurisdictions of College Station and Bryan and including property owned by the Texas A&M University System. In 2012, approximately 200 acres west of Turkey Creek Road were zoned BioCorridor Planned Development District (PDD) to encourage research and development, manufacturing, and warehousing for biomedical and other emerging technology industries in the area. The area surrounding this PDD is anticipated to serve as a secondary or supplementary region that would provide supportive land uses enhancing the development occurring within the BioCorridor District. The BioCorridor Concept Master Plan envisioned a mix of uses in the subject area; thus prompting the request to amend the Future Land Use map to Urban and the zoning to Multi-Family.

Currently, the Comprehensive Plan Future Land Use and Character Map designates the area west of Turkey Creek Road and north of Raymond Stotzer Parkway to be approximately 37 acres of Suburban Commercial, 11 acres of Urban and 198 acre of Business Park, the majority of which is zoned BioCorridor Planned Development District.

The Comprehensive Plan Future Land Use and Character Map designates the area to the east of Turkey Creek Road and west of Harvey Mitchell Parkway to be approximately 23 acres of Suburban Commercial (property already zoned for commercial uses along Harvey Mitchell), 17 acres of Urban, 6 acres of General Commercial, 39 acres to be Texas A&M University (property owned by Texas A&M), 29 acres of Public/Institutional (the City's cemetery), and the largest designation to be 54 acres of Business Park.

The proposed zoning complements and is consistent with the allowable land uses for the Presidential Corridor Gateway District and with the surrounding future land use designations.

2. Whether the uses permitted by the proposed zoning district will be appropriate in the context of the surrounding area: The existing R Rural zoning allows for large lot residential and/or agricultural uses, which is not compatible or supportive of more intense development that is anticipated for the area. Given the property's close proximity to the BioCorridor Planned Development District, this area is expected to contain uses that support the investment plan that has been set in this District. An R Rural designation does not support the planned growth.

The adjacent land uses to the east and south are large lot single- family and unimproved rural tracts varying in size from four to seven acres. The property to the north and west is currently undeveloped and is located within the BioCorridor Planned Development District. The existing rural developments are less dense than the Multi-Family designation prescribes. A Multi-Family development will provide a different character than these existing properties currently provide. However, this is the third rezoning for more intense land uses requested in the area outside of the BioCorridor PDD. The proposed zoning change, while not compatible with the land uses located immediately to the east and south, is compatible as supporting land uses for the Business Park

uses that are anticipated to develop.

- **3. Whether the property to be rezoned is physically suitable for the proposed zoning district:** The proposed land uses permitted through this request are those allowed in MF Multi-Family. The uses are appropriate as they support the uses planned in the BioCorridor PDD and because of the property's proximity to HSC Parkway and Raymond Stotzer Parkway. The property is relatively flat, naturally vegetated, with a small pond. There is no FEMA regulated floodplain onsite.
- 4. Whether there is available water, wastewater, stormwater, and transportation facilities generally suitable and adequate for uses permitted by the proposed zoning district: Water service is provided to properties in this area by the City of College Station and Wellborn Special Utility District (SUD), but additional infrastructure improvements may be needed with further site development to meet minimum fire flow requirements. With future development of the site there will be an offsite extension of waterline required to connect to the existing 18-inch water main on the south side of HSC Pkwy. The extension will be a 12-inch water line constructed with the extension of Atlas Pear Drive that will serve the tract for the portion of the property served by the City of College Station.

Drainage is generally to the south within the Whites Creek Drainage Basin. Currently, there is no FEMA regulated flood plain on site, however, detention will be required with site development. Drainage and other public infrastructure required with the site shall be designed and constructed in accordance with the B/CS Unified Design Guidelines.

The subject tract has frontage to Turkey Creek Road which is designated as a future Major Collector on the Thoroughfare Plan. Turkey Creek Road is currently constructed as two-lane rural section roadway and is under TxDOT jurisdiction as FM 2513. The applicant also anticipates driveway access to Atlas Pear Drive which would be extended with development south from HSC Parkway and be designed as Minor Collector.

A traffic impact analysis (TIA) was performed for the rezoning request and assumed the maximum multi-family density which would allow up to 334 units. The applicant intends to develop over 300 units with construction to be complete in 2021. All intersections studied in the TIA performed at an acceptable level of service except F&B Road at FM 2818 (Harvey Mitchell Parkway) which becomes a failing intersection in the background condition. The intersection of F&B Road at FM 2818 is built out except for the eastbound approach of F&B Road which constrains the capacity of the intersection by having only a single lane. As FM 2818 is a major roadway, the proposed development contributes less than 2% of the traffic to the overall intersection though would provide roughly 20% of the traffic on single-lane eastbound approach in the AM peak hour and about 5% of the eastbound traffic in the PM peak hour. Funding to improve the intersection or widen F&B Road to a future Minor Arterial has not yet been identified.

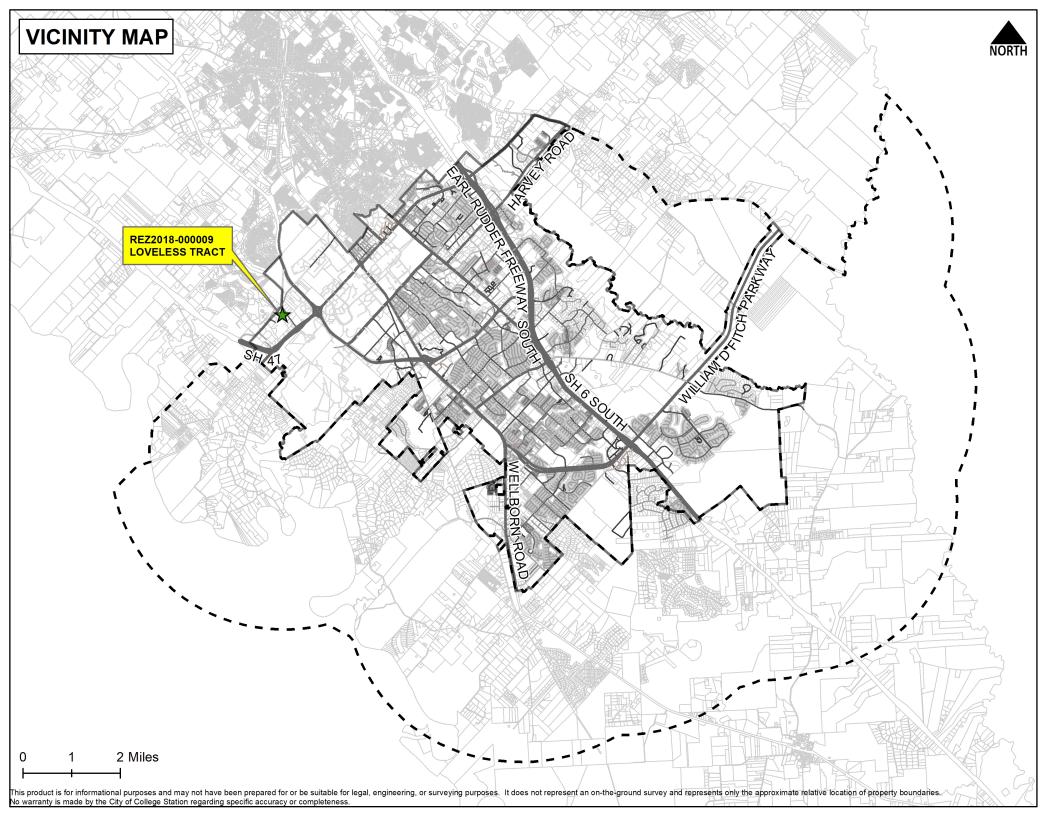
5. The marketability of the property: The property can currently be marketed under the existing R Rural zoning district. However, the applicant has stated that the use is not appropriate or feasible for this property knowing the current development happening on nearby property and the anticipated change in the development pattern for the area.

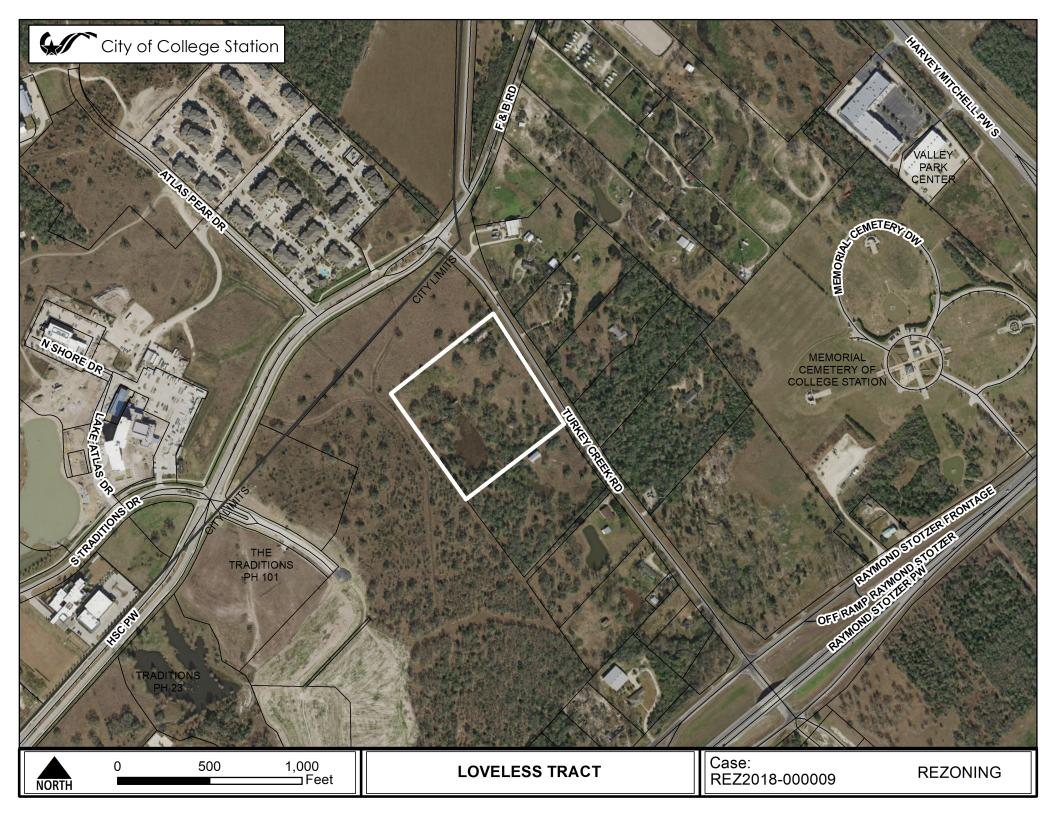
Legal Review: Yes

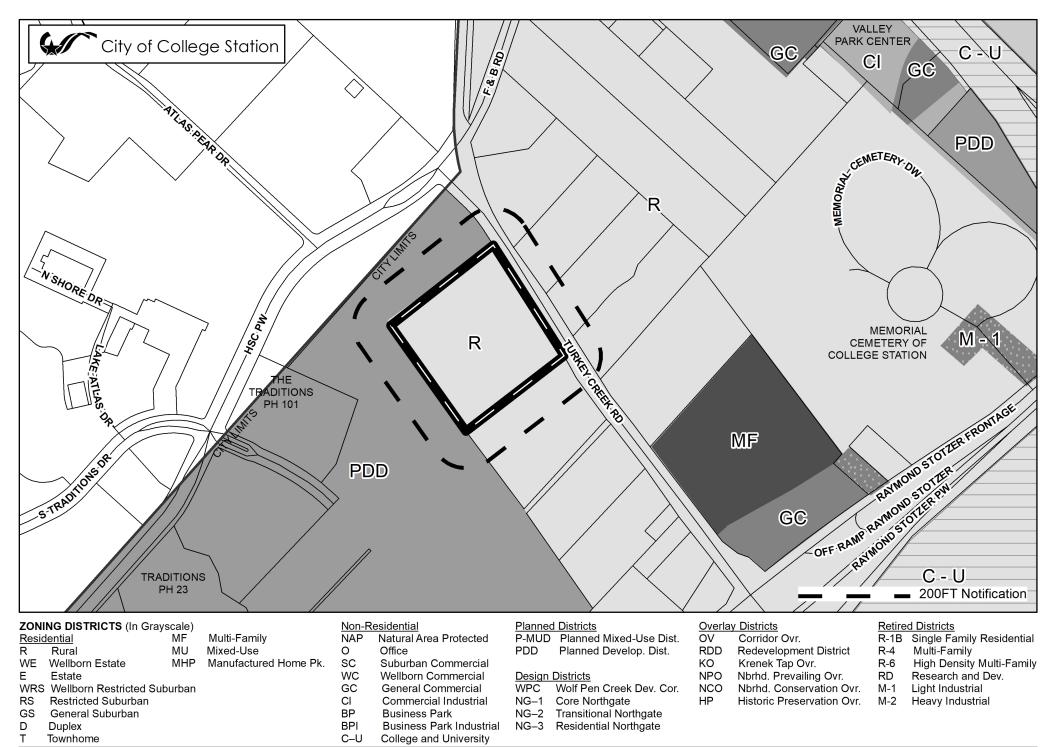
File #: 18-0542, Version: 2

SUPPORTING MATERIALS

- Background Information
 Vicinity Map, Aerial, and Small Area Map
- 3. Rezoning Map
- 4. Ordinance







NORTH 0 500 1,000 Feet

LOVELESS TRACT

Case: REZ2018-000009

REZONING

EXISTINGZoning

PROPOSED Zoning





TRAFFIC IMPACT ANALYSIS

for

Rezoning of Loveless Tract on Turkey Creek Road

in College Station, Texas

Prepared

for

Traditions Acquisition Partnership and Schultz Engineering, LLC

June 7, 2018

Ву





Bradford K. Brimley, Ph.D., P.E.

TRAFFIC IMPACT ANALYSIS

for

Rezoning of Loveless Tract on Turkey Creek Road In College Station, Texas

EXECUTIVE SUMMARY

This Traffic Impact Analysis (TIA) evaluates the vehicular impact of the development of 11.14 acres of land (the Loveless Tract) located on Turkey Creek Road in College Station, Texas. The land is currently zoned Rural. The proposed zoning is Multi-family (MF). This report analyzed the expected access points to the site and the surrounding intersections under the following conditions:

- Existing (2018)
- Background (2021)
- Full Build Proposed (2021)
- Full Build Alternative (2021)

PROPOSED DEVELOPMENT

The development is expected to have two access points. The primary access will be from an extension of Atlas Pear Drive south of Health Science Center Parkway. A secondary access is expected to provide exit-only access to Turkey Creek Road.

A background growth rate of 5.0% was used to forecast traffic volumes to future years. The existing traffic volumes and regional development were utilized to determine trip distributions to and from the site.

CAPACITY ANALYSIS

The analyses for the existing conditions indicate that all intersections currently operate at an acceptable level of service. The analyses for the Background 2021 conditions identified some capacity deficiencies at the intersection of F&B Road at FM 2818. These continue with the Full Build scenarios. All other intersections in the Full Build conditions operate at an acceptable level of service.

RIGHT-TURN LANE ANALYSIS

Based on conditions in the City's General Development Standards, a right-turn deceleration lane is not required at either access as the projected turn volumes do not exceed the City's thresholds.

RECOMMENDATIONS

It is recommended that a left-turn lane from HSC Parkway to Atlas Pear Drive be constructed with the proposed development that mirrors the existing eastbound left-turn lane.

Traffic Impact Analysis – Rezoning of Loveless Tract

Binkley & Barfield, Inc

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INTRODUCTION

This Traffic Impact Analysis (TIA) evaluates the vehicular impact of the development of 11.14 acres of land (the Loveless Tract) located on Turkey Creek Road in College Station, Texas. The land is currently zoned Rural. The proposed zoning is Multi-family (MF). This report analyzed the expected access points to the site and the surrounding intersections under the following conditions:

- Existing (2018)
- Background (2021)
- Full Build Proposed (2021)
- Full Build Alternative (2021)

EXISTING STUDY AREA

A summary of the existing streets and intersections which encompass the study area and the nearby developments are described below.

STREETS AND INTERSECTIONS

The following streets and the intersections they form are included in the analyses of this report.

Health Science Center Parkway (HSC Parkway) and F&B Road connects SH 47 with FM 2818. To the west of Turkey Creek Road, HSC Parkway is a four-lane divided road with a 35-mph speed limit. To the east of Turkey Creek Road, F&B Road is a two-lane undivided road with a 40-mph speed limit. HSC Parkway and F&B Road are identified as a proposed Minor Arterial in the City's Thoroughfare Plan.

FM 60 (Raymond Stotzer Parkway) is a four-lane divided roadway with a two-lane two-way frontage road on the north side and a one-way frontage road on the south side (near the airport). FM 60 is classified in the City's Thoroughfare Plan as a Freeway/Expressway.

Turkey Creek Road is a two-lane undivided road, identified as a Major Collector in the City's Thoroughfare Plan.

Atlas Pear Drive extends north of HSC Parkway, providing access to some of the Traditions development, and specifically to the Hawthorn at Traditions apartments. With the proposed multi-family development, Atlas Pear Drive is to be extended south of HSC Parkway.

Figure 1 provides an aerial photograph of the area, highlighting the subject roads, the location of the proposed development, and the location of ViaSat, a recently completed office complex.

The existing and proposed intersections evaluated in this study are as follows:

- FM 2818 at F&B Road
- Turkey Creek Road at HSC Parkway/F&B Road
- Turkey Creek Road at FM 60 WB
- Turkey Creek Road at FM 60 EB
- HSC Parkway at Traditions Drive
- HSC Parkway at Atlas Pear Drive

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Traffic volumes were counted at each study intersection in March 2018 on a weekday from 7-9 AM and 4-6 PM. The raw traffic count data are provided in the Appendix. The resultant AM and PM Peak Hour volumes for existing weekday traffic are illustrated in **Figure 2**.

AREA DEVELOPMENT

Regionally, the oldest developments in the area include the Easterwood Airport to the south of the area of interest and the Traditions Club to the north. There are multiple existing and planned developments along the Bryan/College Station Biomedical Corridor. Recently completed developments include the Stella Hotel, the MatureWell Lifestyle Center, the Fujifilm/Diosynth laboratories, the iBio complex, and ViaSat.

The ViaSat development opened in April 2018, which occurred after existing traffic counts were obtained for this report. Traffic generated by ViaSat is added to the Existing (2018) traffic as the complex is now open. Trips generated by ViaSat were determined from appropriate land uses and rates in the Trip Generation manual shown in **Table 1** and **Table 2**, respectively.

Table 1. Land Use Data: Recent Developments

Development	ITE	Number of Units	AM Peak Hour		PM Peak Hour	
Development	Code	Number of Offics	Rate/Unit	Trips	Rate/Unit	Trips
ViaSat	710	200 Employees	0.48	96	0.46	92

Table 2. Trip Generation Data: Recent Developments

Davidonment		AM I	Peak (v	ph)		PM Peak (vph)				
Development	Total	% Enter	Enter	% Exit	Exit	Total	% Enter	Enter	% Exit	Exit
ViaSat	96	88%	84	12%	12	92	17%	16	83%	76

The trips generated by ViaSat were distributed to and from the complex to be added to the Existing 2018 traffic and used in the capacity analyses. **Figure 3** depicts the trip distribution for ViaSat. **Figure 4** shows the site-generated trips for ViaSat, combining the trip generation and distribution assumptions.

Figure 5 shows the resultant Existing 2018 Peak Hour volumes after the addition of the Viasat traffic.

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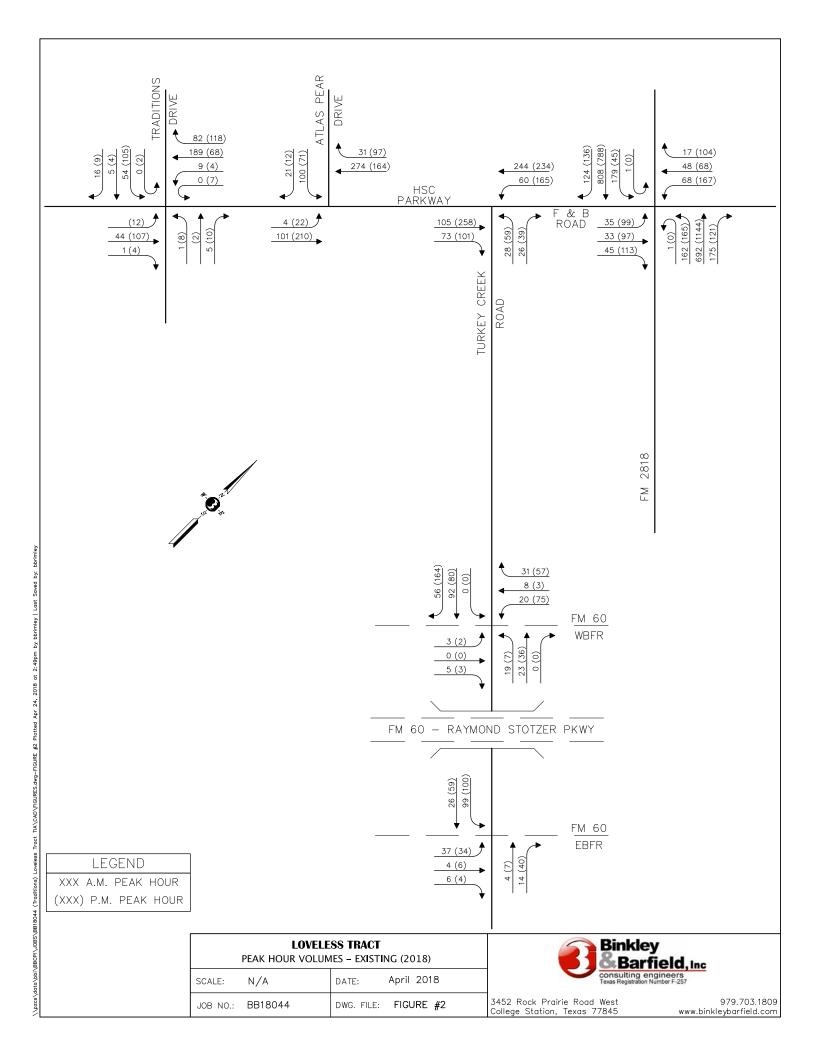


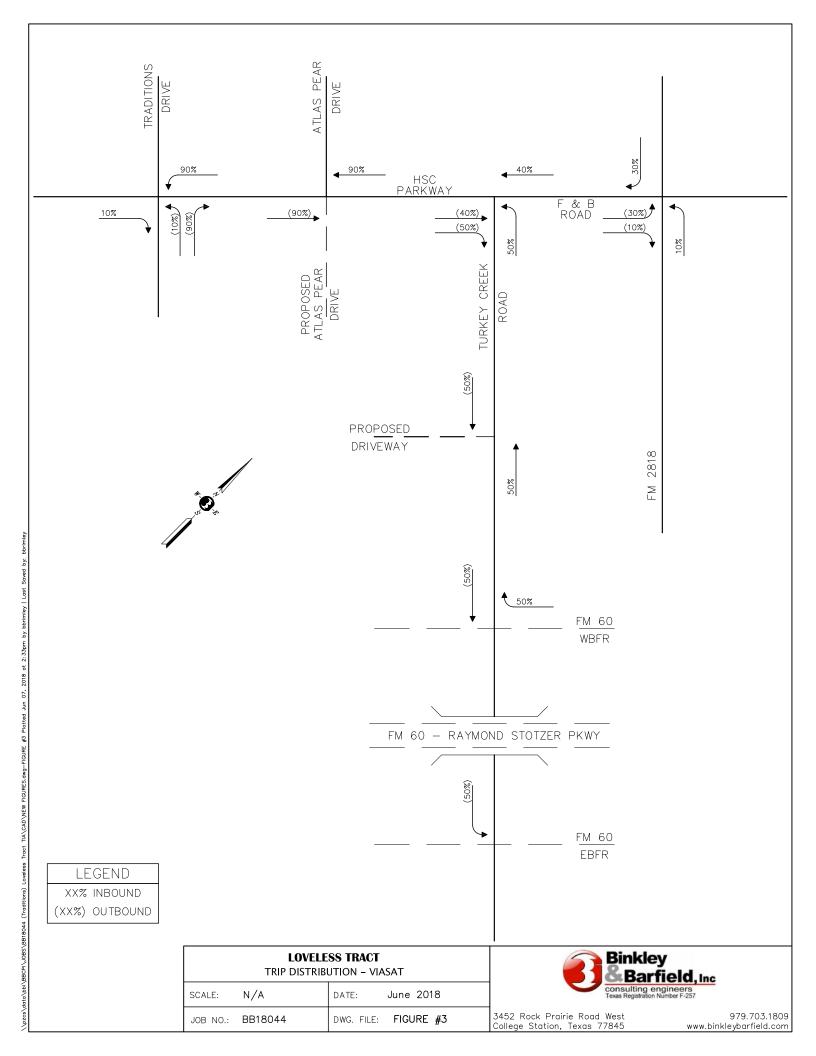
Imagery ©2018 Google, Map data ©2018 Google

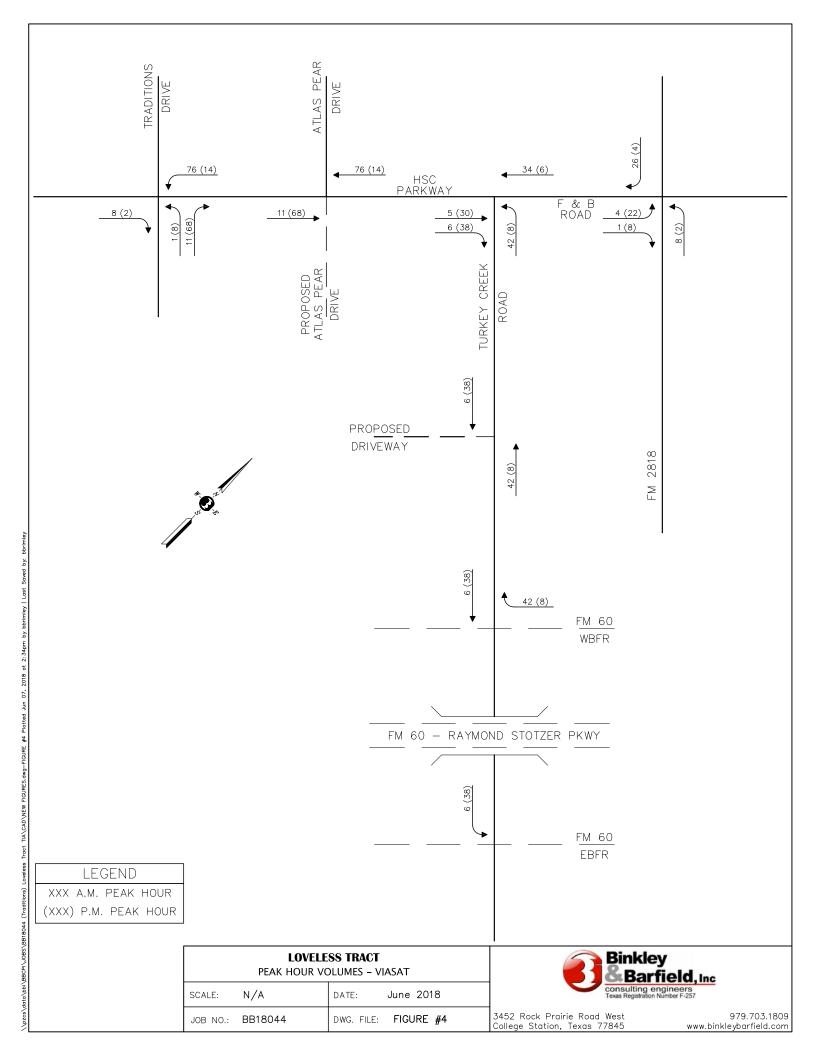
LOVELESS TRACT AERIAL MAP									
SCALE:	N/A	DATE:	April 2018						
JOB NO.:	BB18044	DWG. FILE:	FIGURE #1						

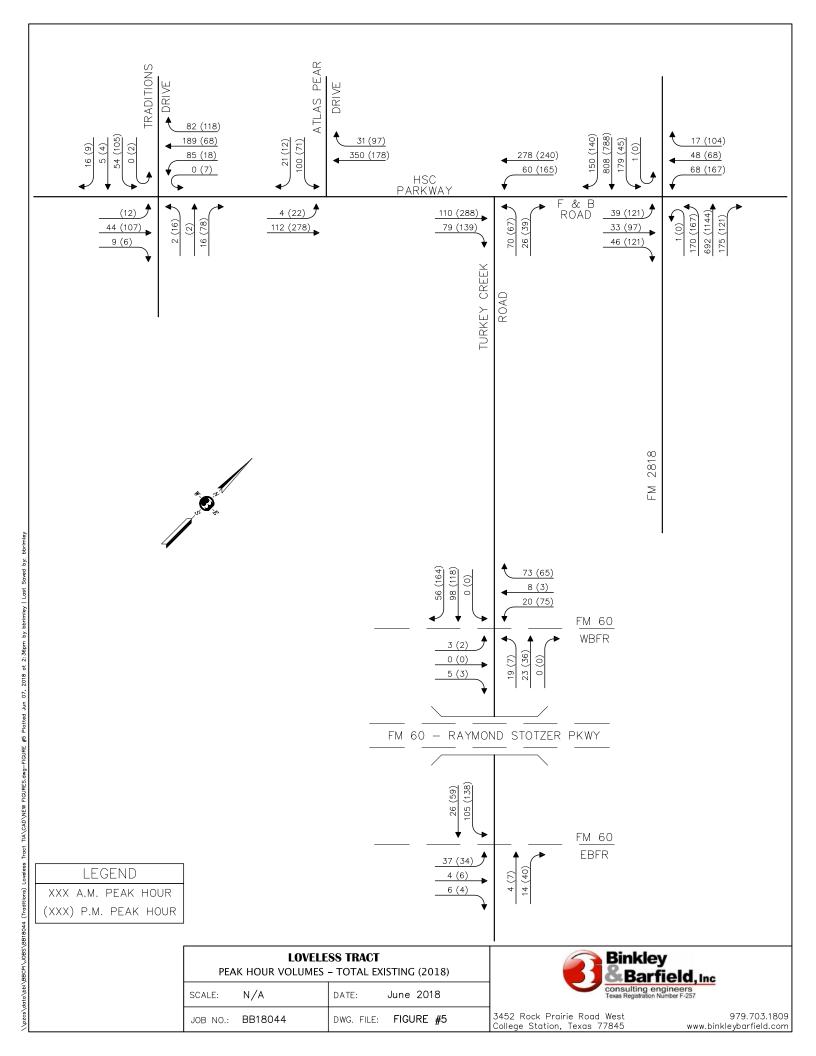


3452 Rock Prairie Road West College Station, Texas 77845 979.703.1809 www.binkleybarfield.com









PROPOSED DEVELOPMENT

This report addresses the rezoning of 11.14 acres for Multi-Family use. An exhibit showing the proposed accesses with an estimate for the number of units is provided in the Appendix. The development is expected to have two access points. The primary access will be from a proposed extension of Atlas Pear Drive south from HSC Parkway. The second access will be on Turkey Creek Road, which is intended to be restricted to exiting vehicles.

LAND USES

The Full Build conditions were analyzed for the year 2021. **Table 1** identifies the relevant Land Use data for the proposed use. The exhibit in the Appendix shows that the development is planned to have 307 apartment units, a density of 27.5 units per acre. The City ordinance permits a density of 30 units per acre for Multi-Family zoning. The analyses of this study will assume a density of 30 units per acre to provide a conservative finding.

Table 3. Land Use Data: Proposed Development

Land Use ITE		Aoros	Units/	Number of	AM Peak Hour		PM Peak Hour	
Land Use	Code Acres		Acre	Units	Rate/Unit	Trips	Rate/Unit	Trips
Multi-family	220	11.14	30 DU	334 DU	0.46	154	0.56	187

TRIP DISTRIBUTION

The trip ends generated by the proposed land use addressed in this study were distributed to and from the site based on its location with respect to area development, nearby roadways, and existing traffic patterns. Two trip distributions are shown. The first is consistent with the exhibit in the Appendix that shows the main access on Atlas Pear Drive and an exit-only access on Turkey Creek Road. **Figure 6** illustrates this proposed trip distribution. The second distribution shows full access to Turkey Creek Road in addition to the proposed access on Atlas Pear Drive. This alternative distribution is shown in **Figure 7**.

TRIP GENERATION

For this site, the AM and PM Peak Hour trips are determined from the Institute of Transportation Engineers' (ITE) 10th edition Trip Generation Manual. **Table 2** summarizes the associated trip generation data and the calculated trips that are anticipated to be generated by the proposed development. Information about the ITE data used to develop **Table 2** is included in the Appendix.

Table 4. Trip Generation Data: Proposed Development

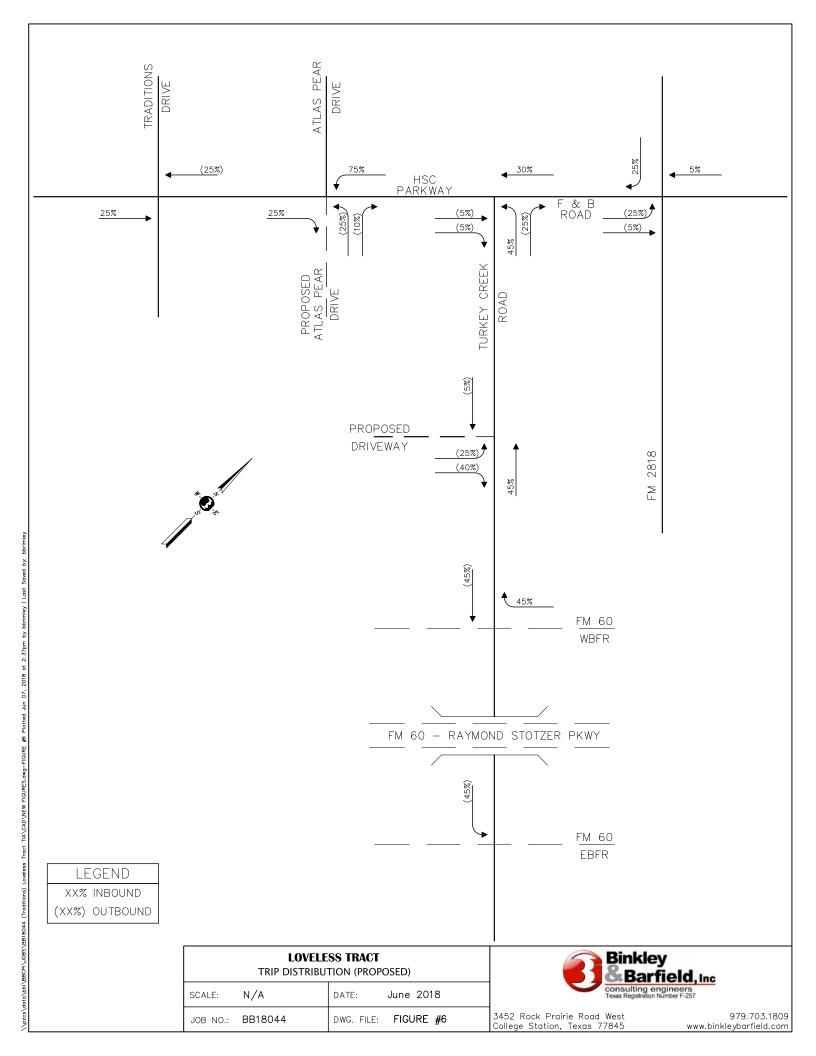
Landlia	AM Peak (vph)						PM Peak (vph)			
Land Use	Total	% Enter	Enter	% Exit	Exit	Total	% Enter	Enter	% Exit	Exit
Multi-family	154	23%	35	77%	118	187	63%	118	37%	69

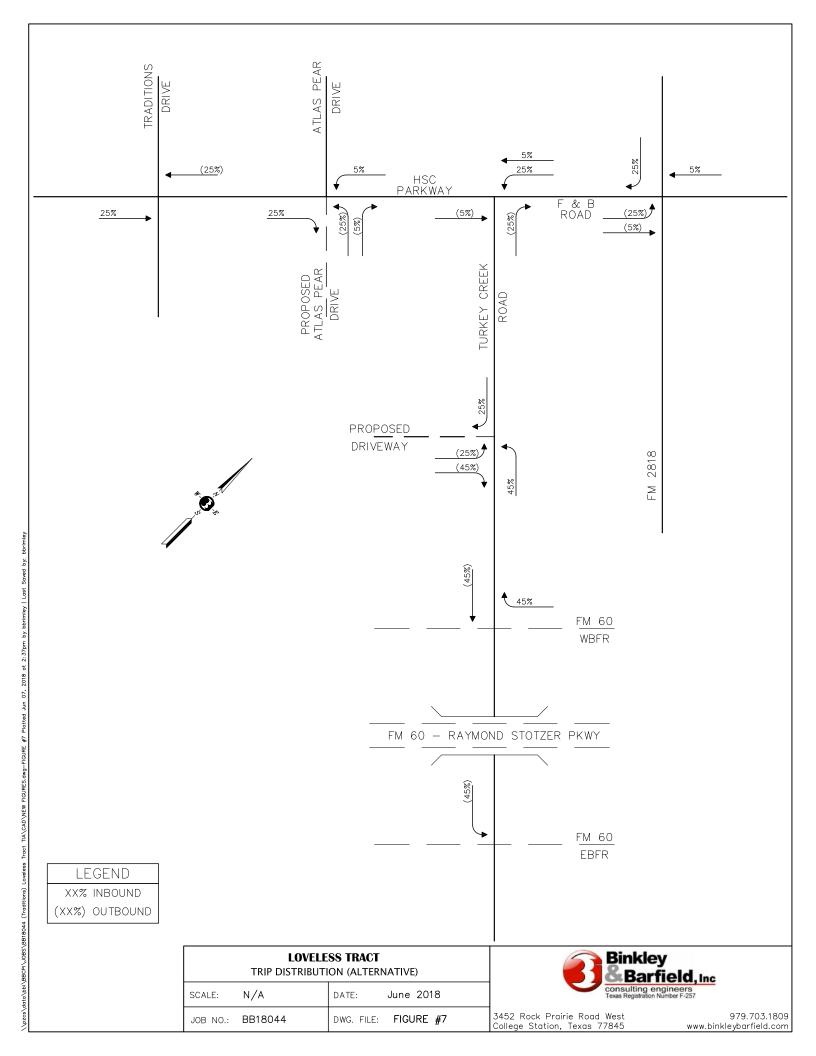
PROJECTED TRAFFIC VOLUMES

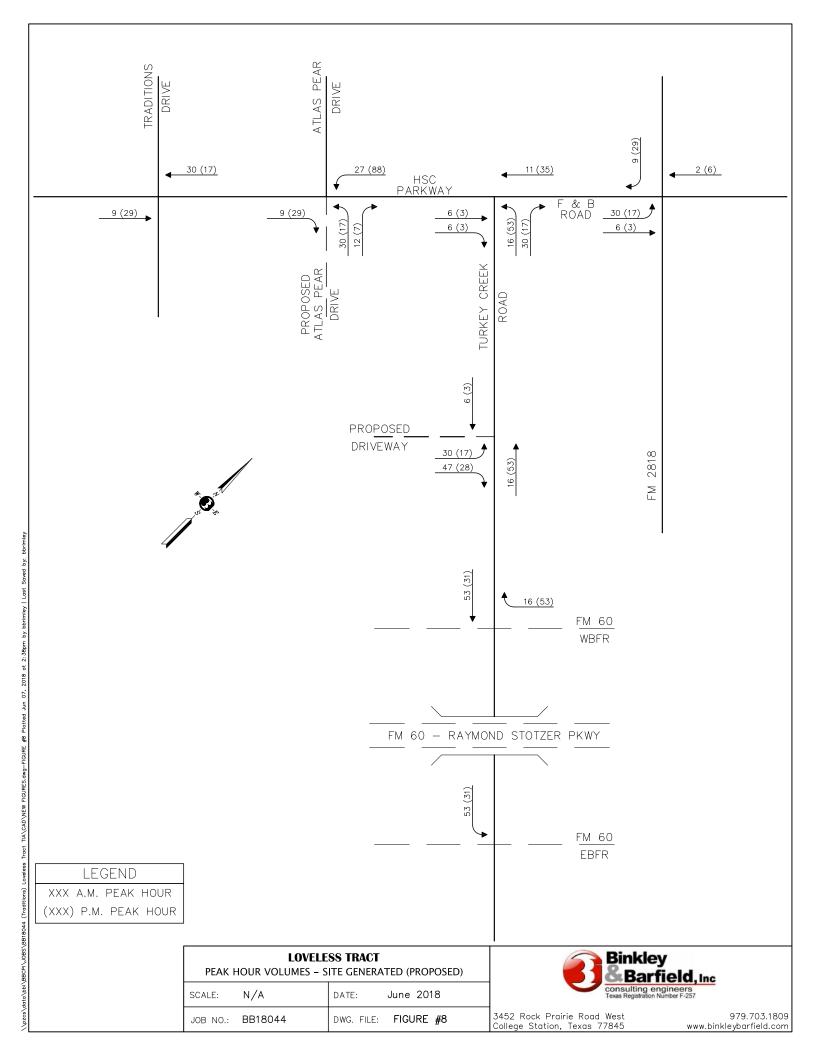
SITE TRAFFIC VOLUMES

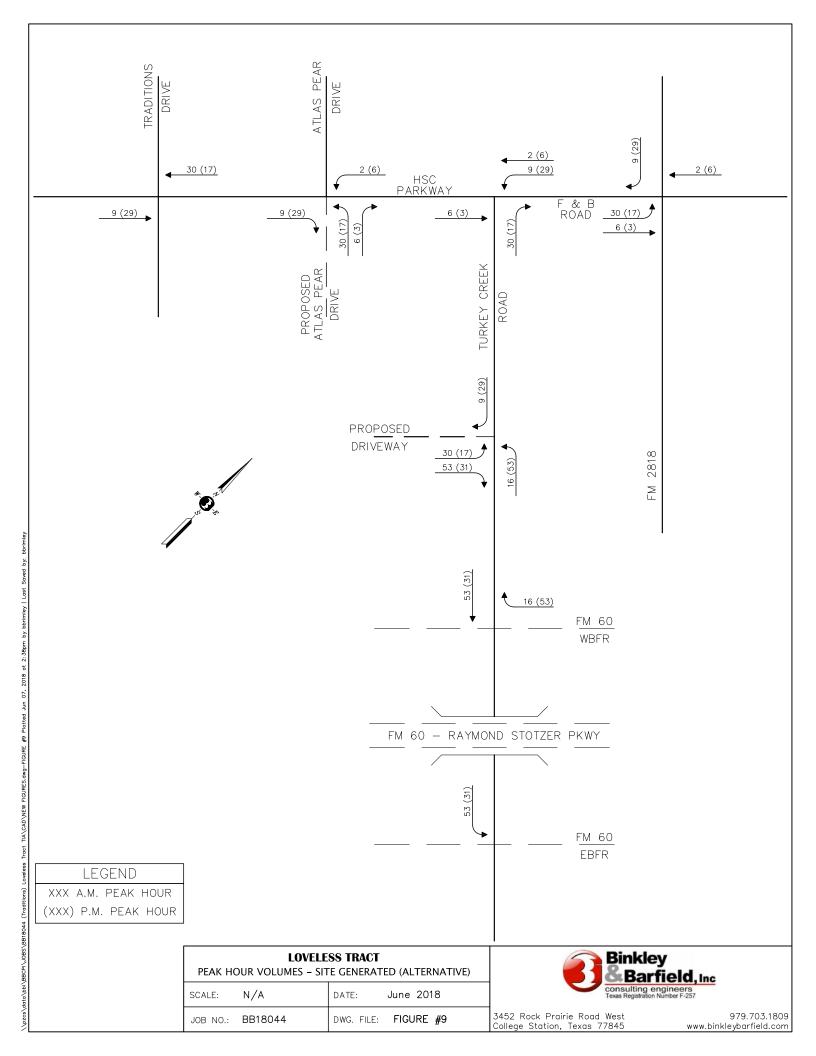
For this study, "site volumes" represent the traffic expected to be generated by the proposed site. **Figure 8** depicts the site volumes distributed to and from the site during the AM and PM Peak Hours of a typical weekday consistent with the **Figure 6** intended distribution. **Figure 9** depicts the site volumes for the alternative access scenario.

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BACKGROUND TRAFFIC VOLUMES

For this study, "background volumes" represent the traffic expected to occur along the area streets and roadways because of the increase in traffic volumes due to normal growth in the area. This study applied an annual growth factor of 5%, consistent with rates seen for similar locations with undeveloped land in the area and determined by evaluating historic traffic volumes from the Texas Department of Transportation's (TxDOT) Statewide Planning Map.

Figure 10 depicts the background traffic determined by increasing the existing traffic at a 5% growth factor to the year 2021.

TOTAL TRAFFIC VOLUMES

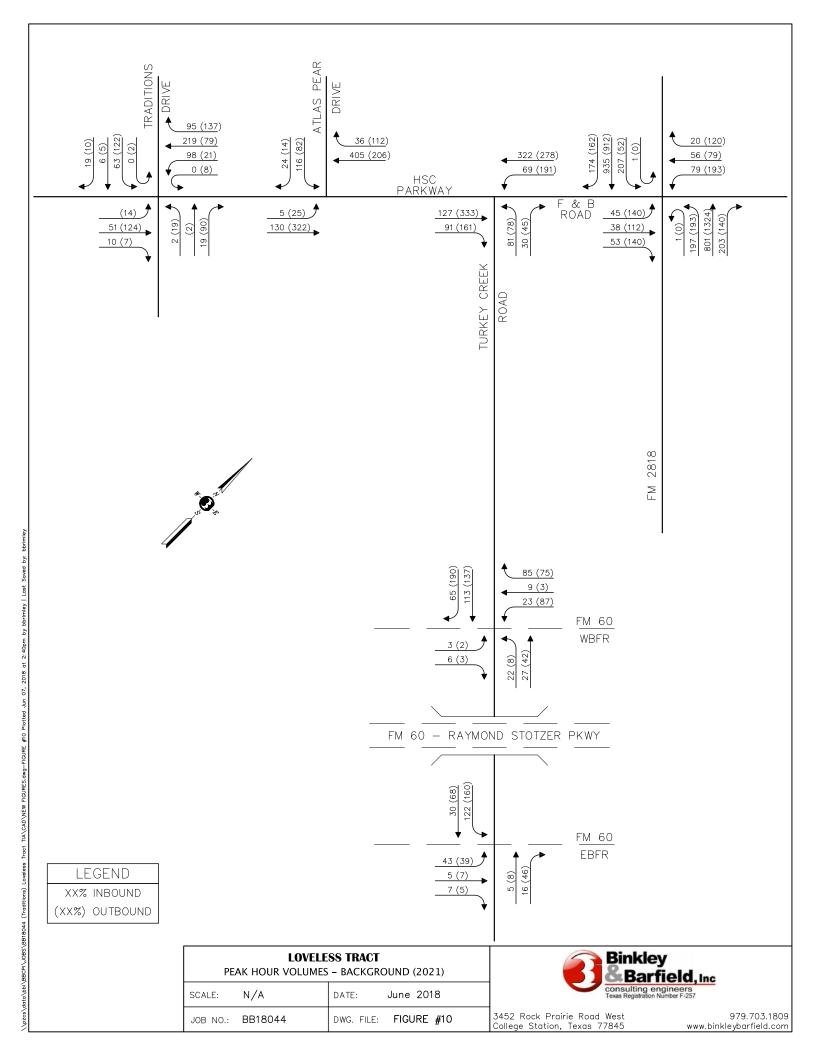
Figure 11 displays the proposed "total volumes" anticipated to occur within the study area for the Full Build 2021 conditions. The total proposed volumes depicted combine the site generated volumes in **Figure 8** and the background volumes in **Figure 10**.

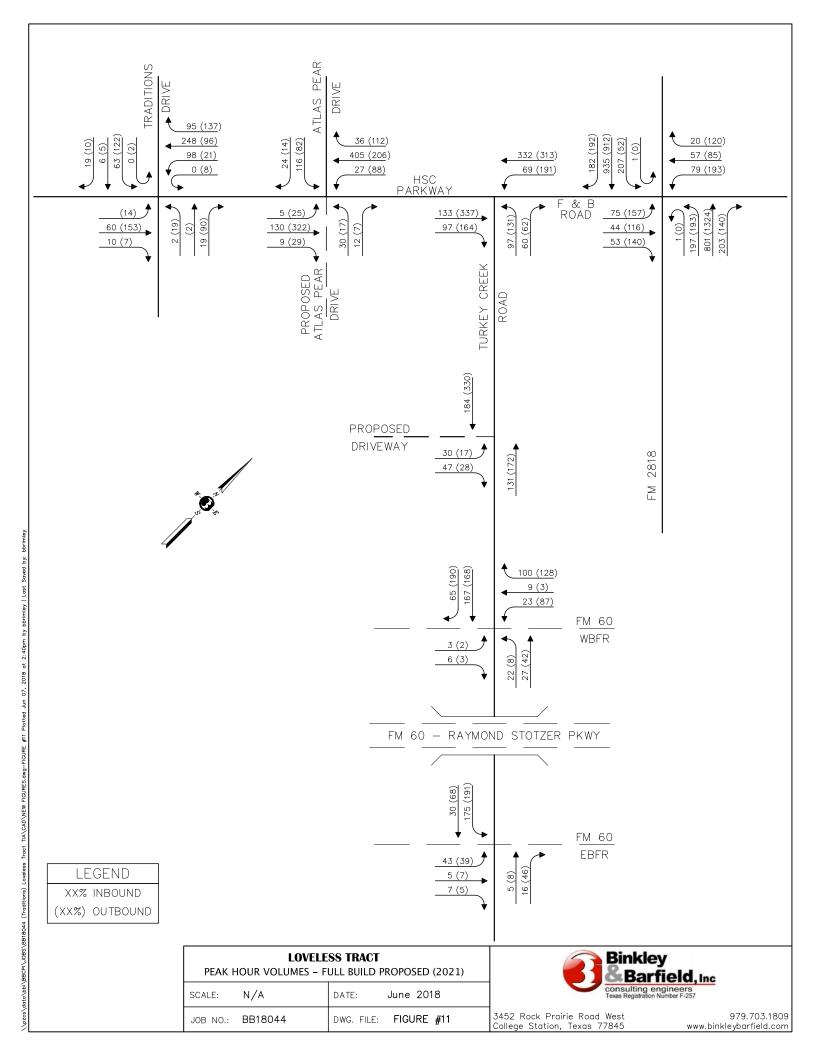
Figure 12 displays the total volumes for the alternative scenario, combining the alternative sitegenerated volumes of **Figure 9** with the background volumes in **Figure 10**.

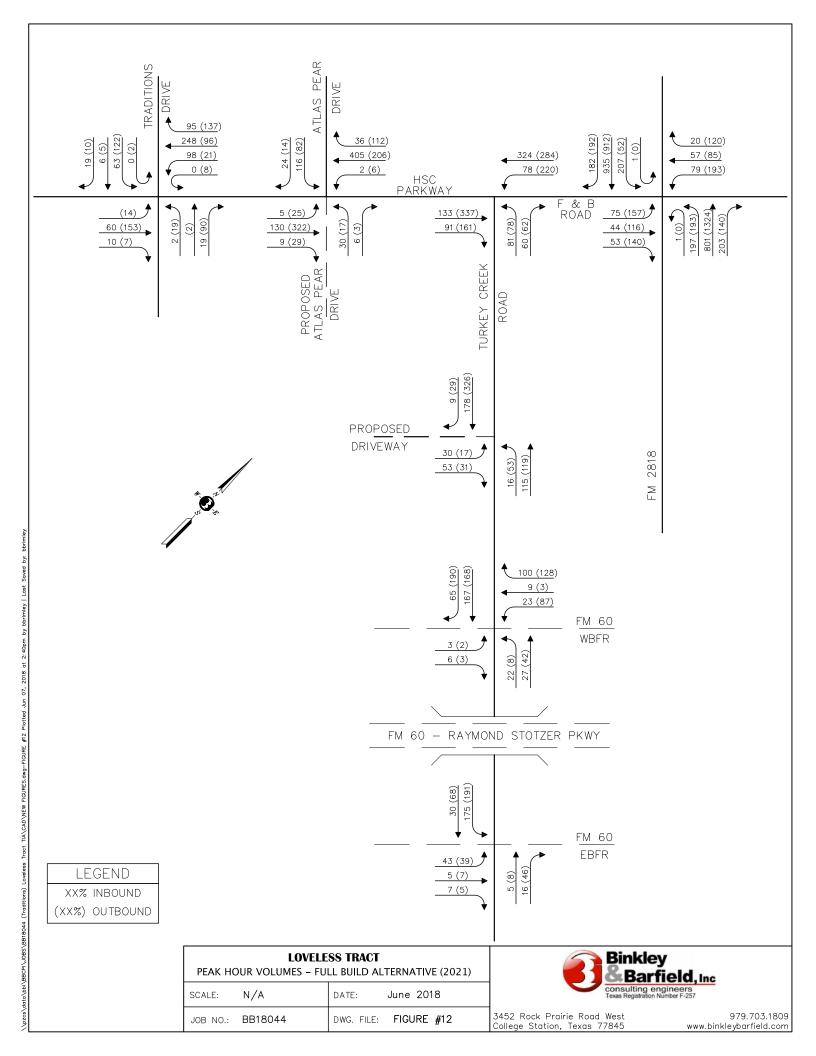
The city's Unified Development Ordinance (Section 7.13) states that developments that contribute 5% or more of the traffic at locations failing to meet level of service "D" should be mitigated by the applicant (developer). **Table 5** indicates the proportion of Peak Hour traffic in the full build conditions that is the attributed to the proposed development. The information in **Table 5** is based on the traffic volumes shown in **Figure 8** and **Figure 11** for the Proposed condition and **Figure 9** and **Figure 12** for the Alternative scenario.

Table 5. Proportion of Peak Hour Traffic Generated by the Development

	Prop	osed	Alternativ	e Access
Location	AM	PM	AM	PM
FM 2818 at F&B Road	1.6%	1.5%	1.6%	1.5%
Turkey Creek Road at HSC Parkway/F&B Road	8.6%	9.4%	6.0%	4.9%
Turkey Creek Road at FM 60 WB	16.4%	13.3%	16.4%	13.3%
Turkey Creek Road at FM 60 EB	19.0%	8.5%	19.0%	8.5%
HSC Parkway at Traditions Drive	6.2%	6.8%	6.2%	6.8%
HSC Parkway at Atlas Pear Drive	9.7%	15.7%	6.1%	6.9%







INTERSECTION CAPACITY ANALYSIS

Level of Service (LOS) analyses of the traffic operations were performed at the existing intersections and the proposed access points. Analyses of the intersections were conducted utilizing Synchro software. The results of the capacity analyses for the intersections with the resulting delay and levels of service values are summarized by approach in the following tables:

- Table 6. AM Peak Hour Level of Service
- Table 7. PM Peak Hour Level of Service

Copies of the Synchro computer printouts and a description of the various levels of service have been included in the Appendix. Typically, the desirable levels of service are "A" through "D." Levels of Service "E" and "F" are undesirable.

In the analyses, it was assumed that a westbound left-turn lane from HSC Parkway onto Atlas Pear Drive is constructed to match the eastbound left-turn lane.

2018 EXISTING

With the current traffic on the existing lane configurations, all intersections operate at an acceptable level of service. The eastbound approach of F&B Road at FM 2818 operates at LOS E during the PM Peak Period. No mitigation is proposed as the intersection operates at LOS D with an average delay of 45 seconds per vehicle.

2021 BACKGROUND

The 2021 Background analysis assumed a 5.0% background growth per year and additional traffic from ViaSat. The only intersection with operational concerns is F&B Road at FM 2818, which is forecasted to operate at LOS E during the PM Peak Hour. It is apparent that the eastbound approach, with only one lane, is what limits capacity most at this intersection.

2021 FULL BUILD PROPOSED

The 2021 Full Build scenario analyzed traffic conditions with the 11.14-acre Multi-Family development added to the background traffic. During the PM Peak Hour, the intersection of F&B Road at FM 2818 continues to operate at LOS E. Additionally, the westbound approach of F&B Road/ HSC Parkway at Turkey Creek Road operates at LOS E; however, the intersection overall operates at LOS C. As this intersection is all-way stop controlled, the poor level of service on that one approach is not a safety concern. There is spare capacity at the intersection and the volume to capacity (v/c) ratio for the westbound approach is 0.88.

As shown in Table 5, the proposed development contributes less than 2% of the total Peak Hour traffic at the intersection of F&B Road at FM 2818. Mitigation for level of service lower than "D" is required by ordinance only when the development's traffic exceeds 5% of the total traffic. Therefore, mitigation at the intersection is not required. It is worth noting that, as a proposed Minor Arterial, the future eastbound lane configuration when F&B Road is widened will add capacity and significantly improve the intersection's performance. There is not a known time when F&B Road will be widened.

2021 FULL BUILD ALTERNATIVE

For the Alternative scenario, multiple intersections experience the same traffic volumes and therefore the same delay and level of service. The most notable difference is Turkey Creek Road at HSC Parkway, which improves in the Alternative scenario because the full-access driveway on Turkey Creek Road allows vehicles to bypass the all-way stop on HSC Parkway.

Table 6. AM Peak Hour Level of Service

0	Ea	stbou	ınd	We	estbou	nd	No	rthbou	ınd	Sou	uthbo	und	15.7
Scenario	LT	тн	RT	LT	тн	RT	LT	ТН	RT	LT	ТН	RT	INT
				F&E	3 Road	l at FM	2818					<u> </u>	
Eviating 2019		D			D			В			С		С
Existing 2018		50.4			47.4			14.3			20.0		20.5
Background		Е			Е			С			С		С
2021		76.9			69.2			24.0			30.5		32.1
Full Build		F			E			С			С		D
Proposed 2021		81.6			69.8			27.6			34.1		36.1
Full Build		F			Е			С			С		D
Alternative 2021		81.6			69.8			27.6			34.1		36.1
			Turk	ey Cre	ek Ro	ad at F	ISC Pa	ırkway	/				
Existing 2018		Α		E	3	-		Α			-		В
Exioting 2010		8.5		12	2.2	-		9.4			-		10.6
Background		Α		E		-		Α			-		В
2021		8.9		14	.5	-		9.9			-		12.1
Full Build		Α		C		-		В			-		В
Proposed 2021		9.3		16		-		10.7			-		13.0
Full Build		Α		C		-		В			-		В
Alternative 2021		9.2		15	.7	-		10.3			-		12.8
		ı	HS	C Park	way a	t Atlas	Pear I	Drive		Т			
Existing 2018	Α	-	-		-			-			В		-
	8.2	-	-		-			-			14.8		-
Background	Α	-	-		-			-			С		-
2021	8.4	-	-		-			-			17.5		-
Full Build	Α	-	-	Α		-		В			С		-
Proposed 2021	8.4	-	-	7.6		-		12.2			19.8		-
Full Build Alternative 2021	A	-	-	Α	,	-		В			C		-
Alternative 2021	8.4	-	-	7.5				11.9			17.8		-
			Tra	dition		e at HS	C Parl			I	_		
Existing 2018		Α			Α			Α			Α		Α
		8.1			8.7			8.0			9.5		8.7
Background 2021		A			A			A			A		A
		8.3			9.1			8.2			9.9		9.1
Full Build Proposed 2021		A			A			A			B		A
		8.4			9.2			8.3			10.0		9.2
Full Build Alternative 2021		A			A			A			B		A
Allemative 2021		8.4			9.2			8.3			10.0		9.2

Table 6. AM Peak Hour Level of Service (Continued)

Saar'-	Eas	stbour	nd	W	estbour	nd	Nor	thbou	und	Sou	ıthbou	nd	INT
Scenario	LT	тн	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT	INT
			Tu	rkey C	reek Ro	ad at I	FM 60 E	BFR					
Friedra v 0040		Α			-			Α			Α		Α
Existing 2018		8.6			-			7.3			8.8		8.6
Dealeround 2021		Α			-			Α			Α		Α
Background 2021		8.8			-			7.4			9.0		8.8
Full Build Proposed		Α			-			Α			В		Α
2021		9.0			-			7.6			10.1		9.7
Full Build		Α			-			Α			В		Α
Alternative 2021		9.0			-			7.6			10.1		9.7
			Tur	key Cı	eek Ro	ad at F	M 60 W	/BFR					
Existing 2018		Α			Α			Α			Α		Α
Existing 2016		7.4			7.8			8.1			8.4		8.1
Background 2021		Α			Α			Α			Α		Α
Dackground 2021		7.5			8.1			8.2			8.8		8.5
Full Build Proposed		Α			Α			Α			Α		Α
2021		7.7			8.4			8.4			9.7		9.1
Full Build		Α			Α			Α			Α		Α
Alternative 2021		7.7			8.4			8.4			9.7		9.1
		•	Turkey	/ Creek	Road a	at Prop	osed D	rivew	ay				
Full Build Proposed		В			-			-			-		1
2021		10.3			-			-			-		ı
Full Build		В			-		Α		-		-		1
Alternative 2021		10.4			-		7.7		-		-		-

Table 7. PM Peak Hour Level of Service

0	Ea	stboun	ıd	We	estbou	nd	No	rthbo	und	Soi	uthbou	ınd	INIT
Scenario	LT	TH	RT	LT	TH	RT	LT	тн	RT	LT	ТН	RT	INT
				F&B R	oad at	FM 281	8				-		_
Eviating 2019		F			D			D			D		D
Existing 2018		81.1			52.1			41.4			44.9		48.1
Background 2021		F			Е			Е			Е		Е
Background 2021		115.5			77.6			56.5			58.6		66.0
Full Build Proposed		F			E			Е			Ε		E
2021		120.5			77.5			62.7			59.2		69.8
Full Build Alternative		F			E			Е			Е		E
2021		120.5			77.5			62.7			59.2		69.8
		•	Turke	y Creek	Road a	at HSC	Parkw	ау					
Existing 2018		В		С	;	-		Α			-		В
Existing 2010		11.5		16	.4	-		10.4			-		13.5
Background 2021		В		С	;	-		В			-		С
Dackground 2021		13.6		23	.9	-		11.4			-		17.8
Full Build Proposed		С		E		-		В			-		С
2021		16.0		37	.5	-		13.9			-		24.7
Full Build Alternative		В		D)	-		В			-		С
2021		14.4		30	.6	-		11.9			-		21.2
			HSC	Parkwa	y at At	las Pea	r Driv	е					
Existing 2018	Α	-	-		-			-			В		-
Existing 2010	8.0	-	-		-			-			14.4		-
Background 2021	Α	-	-		-			-			С		-
Background 2021	8.2	-	-		-			-			16.5		-
Full Build Proposed	Α	-	-	Α		-		С			D		-
2021	8.2	-	-	8.5		-		17.9			26.1		-
Full Build Alternative	Α	-	-	Α		-		В			С		-
2021	8.2	-	-	8.2		-		14.7			17.0		-
			Trad	itions D	rive at	HSC P	arkwa	y					
Existing 2018		Α			Α			Α			Α		Α
Exioting 2010		8.7			8.7			8.7			10.2		9.0
Background 2021		Α			Α			Α			В		Α
Duonground 2021		9.1			9.1			9.1			10.8		9.5
Full Build Proposed		Α			Α			Α			В		Α
2021		9.4			9.3			9.4			11.1		9.7
Full Build Alternative		Α			Α			Α			В		Α
2021		9.4			9.3			9.4			11.1		9.7

Table 7. PM Peak Hour Level of Service (Continued)

0	Eas	stbour	nd	W	estbou	nd	Noi	rthbou	ınd	So	uthbou	nd	13.17
Scenario	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT	INT
	<u> </u>		Tur	key Cr	eek Ro	ad at I	FM 60 E	BFR	<u> </u>				
Full-time 0040		Α			-			Α			Α		Α
Existing 2018		8.7			-			7.6			8.7		8.5
Bookground 2024		Α			-			Α			Α		Α
Background 2021		9.0			-			7.7			9.1		8.9
Full Build Proposed		Α			-			Α			Α		Α
2021		9.1			-			7.8			9.7		9.3
Full Build Alternative		Α			-			Α			Α		Α
2021		9.1			-			7.8			9.7		9.3
			Turk	key Cr	eek Ro	ad at F	M 60 W	/BFR					
Existing 2018		Α			Α			Α			Α		Α
Existing 2016		7.7			8.7			8.2			9.5		9.1
Background 2021		Α			Α			Α			В		Α
Background 2021		7.9			9.2			8.4			10.4		9.8
Full Build Proposed		Α			Α			Α			В		В
2021		8.1			9.9			8.7			11.5		10.7
Full Build Alternative		Α			Α			Α			В		В
2021		8.1			9.9			8.7			11.5		10.7
		T	urkey	Creek	Road	at Prop	osed C	Privew	ay				
Full Build Proposed		В			-			-			-		-
2021		11.5			-			-			-		ı
Full Build Alternative		В			-		Α		-		-		1
2021		12.0	•		-		8.2		-		-		-

RIGHT-TURN LANE ANALYSIS

The City of College Station's General Development Standards states the following:

"A right-turn deceleration lane with storage length plus taper may be required for any access with a projected peak hour right-turn ingress turning volume greater than 50 vehicles per hour (vph). If the posted speed is greater than 40 mph, a right-turn deceleration lane and taper may be required for any access with a projected peak hour ingress turning volume greater than 25 vph." – Article 7 Section 7.3.I

Based on the City's General Development Standards, a right-turn deceleration lane from HSC Parkway (or from Turkey Creek Road in the Alternative Scenario) is required as the projected right turn volumes are below the City's threshold.

SUMMARY OF FINDINGS

The traffic impact analysis results are summarized in the following paragraphs.

CAPACITY ANALYSIS

The capacity analyses for the existing conditions indicate that all intersections currently operate at an acceptable level of service. The analyses for the Background 2021 conditions identified some capacity deficiencies at the intersection of F&B Road at FM 2818. These continue to exist with the Full Build scenarios. Because the traffic added to the intersection in the Full Build scenarios is less than 2% of the total traffic at the intersection, no mitigation is required.

Given the current geometry of the F&B Road/FM 2818 intersection, with one eastbound lane that serves all three movements (left, through, and right), the most basic mitigation is an eastbound dedicated right-turn lane. Additional improvements can be achieved with a dedicated left-turn lane. Though the time when F&B Road will be widened is unknown at this time, it can be expected to occur in the future as the road is listed as a Minor Arterial. If these possible mitigations do not occur before then, they can be expected at least with the widening.

The analyses of the full build conditions indicate that all other intersections will operate at an acceptable level of service.

RIGHT-TURN ANALYSIS

Based on the City's General Development Standards, a right-turn deceleration lane is not necessary at either of the proposed access points, as the projected right turn volumes are below the City's right-turn lane thresholds.

RECOMMENDATIONS

It is recommended that a left-turn lane from HSC Parkway to Atlas Pear Drive be constructed with the proposed development to correspond with the existing eastbound left-turn lane.

Operations at the intersection of F&B Road at FM 2818 should be monitored to determine when future improvements will be necessary to bring the performance of the intersection to an acceptable level. The undeveloped land along the Biomedical Corridor will lead to high growth rates in the area. This will necessitate improvements at the F&B Road/FM 2818 intersection, and possibly along the existing two-lane portion of F&B Road.

CLOSING

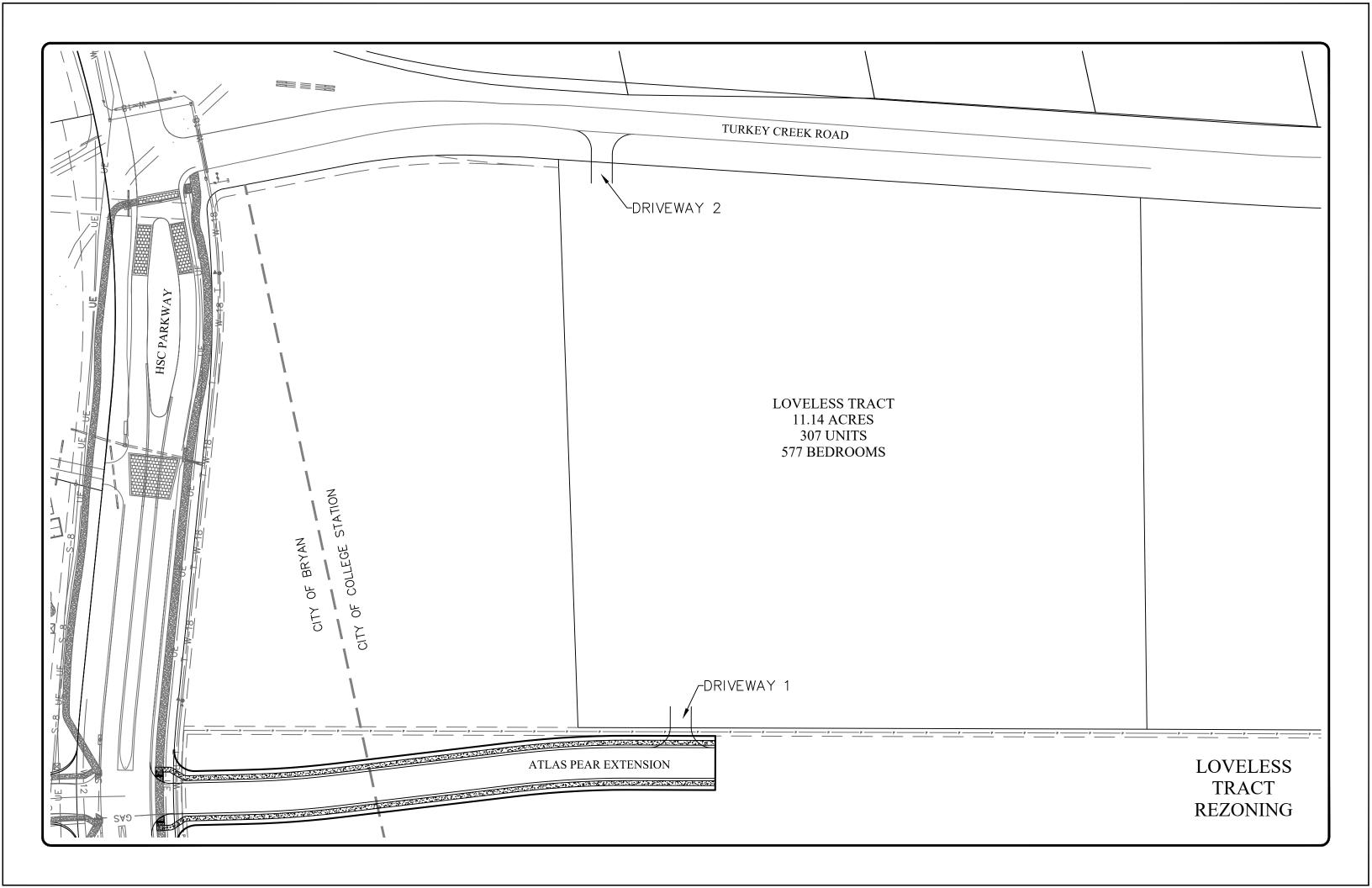
The findings of this TIA are based on evaluations of information available about the potential development of the site and various assumptions as discussed in this study. If the actual future conditions are different than the forecasts of this study, the true performance of the intersections will be different and may justify further analysis. Should any questions arise concerning this report or its analyses, please do not hesitate to contact our office.

Binkley & Barfield, Inc.

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APPENDIX

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SYNCHRO OUTPUT SHEETS	
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FM 2818 at F&B Road 3/21/2018

Ctout																				
Start Time		Eastk	ound			West	oound			North	bound			South	bound		15-min	Hourly	Hourly	
Time	Left	Thru	Right	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right	U-Turn	Total	Total	Rank	PHF
7:00 AM	13	8	2	0	12	10	6	0	37	168	15	0	14	201	12	0	498			
7:15 AM	10	9	7	1	27	18	2	1	34	241	40	0	31	221	23	0	665			
7:30 AM	13	16	14	0	19	13	4	0	45	257	58	0	74	211	35	0	759			
7:45 AM	11	10	13	0	10	11	4	0	49	218	50	0	49	210	42	1	678	2600	1	0.85639
8:00 AM	6	2	9	0	19	15	2	0	41	80	34	1	33	185	21	0	448	2550	2	0.839921
8:15 AM	5	5	9	0	20	9	7	0	27	137	33	0	23	202	26	0	503	2388	3	0.786561
8:30 AM	5	7	9	1	13	18	9	0	32	130	12	3	22	164	30	0	455	2084	4	0.768437
8:45 AM	8	12	4	1	10	18	3	0	16	173	27	0	15	153	19	0	459	1865	5	0.926938
4:00 PM	13	11	24	0	42	17	12	0	29	215	11	0	11	203	18	0	606			
4:15 PM	18	11	22	0	27	14	16	0	34	195	12	0	14	204	25	0	592			
4:30 PM	15	10	23	0	46	16	17	0	43	198	30	0	9	196	32	0	635			
4:45 PM	21	13	16	0	33	14	12	0	50	214	17	0	8	167	35	0	600	2433	5	0.957874
5:00 PM	33	22	39	0	75	24	36	0	46	267	29	0	13	202	41	0	827	2654	4	0.802297
5:15 PM	29	23	34	0	35	12	28	0	48	312	29	0	12	228	36	0	826	2888	3	0.873035
5:30 PM	17	13	16	0	33	17	24	0	41	312	33	0	9	203	34	0	752	3005	2	0.908404
5:45 PM	20	39	24	0	24	15	16	0	30	253	30	0	11	155	25	0	642	3047	1	0.9211

Turkey Creek at FM 60 EB 3/22/2018

Ctant																				
Start Time		Eastl	oound			Westl	bound			North	bound			South	bound		15-min	Hourly	Hourly	
Time	Left	Thru	Right	U-Turn	Total	Total	Rank	PHF												
7:00 AM	9	1	0	0	0	0	0	0	0	1	2	0	14	4	0	0	31			
7:15 AM	9	0	0	0	0	0	0	0	0	1	1	0	11	5	0	0	27			
7:30 AM	10	1	0	0	0	0	0	0	0	0	4	0	18	7	0	0	40			
7:45 AM	18	3	0	0	0	0	0	0	0	1	2	0	35	4	0	0	63	161	5	0.638889
8:00 AM	6	0	2	0	0	0	0	0	0	1	2	0	19	9	0	0	39	169	3	0.670635
8:15 AM	3	0	4	0	0	0	0	0	0	2	6	0	27	6	0	0	48	190	1	0.753968
8:30 AM	6	0	1	0	0	0	0	0	0	2	6	0	11	10	0	0	36	186	2	0.738095
8:45 AM	1	1	0	0	0	0	0	0	0	3	8	0	12	14	0	0	39	162	4	0.84375
4:00 PM	8	3	9	0	0	0	0	0	0	1	14	0	21	10	0	0	66			
4:15 PM	2	2	0	0	0	0	0	0	0	1	8	0	24	7	0	0	44			
4:30 PM	9	0	0	0	0	0	0	0	0	2	11	0	9	7	0	0	38			
4:45 PM	8	2	1	0	0	0	0	0	0	1	7	0	22	12	0	0	53	201	5	0.761364
5:00 PM	13	0	0	0	0	0	0	0	0	1	11	0	35	16	0	0	76	211	4	0.694079
5:15 PM	4	2	1	0	0	0	0	0	0	3	12	0	23	14	0	0	59	226	3	0.743421
5:30 PM	9	2	2	0	0	0	0	0	0	2	10	0	20	17	0	0	62	250	1	0.822368
5:45 PM	6	1	3	0	0	0	0	0	0	1	9	0	16	16	0	0	52	249	2	0.819079

Turkey Creek at FM 60 WB 3/22/2018

Ctaut																	1			
Start Time		Eastl	oound			West	bound			North	bound			South	bound		15-min	Hourly	Hourly	
111110	Left	Thru	Right	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right	U-Turn	Total	Total	Rank	PHF
7:00 AM	3	0	1	0	7	1	7	0	2	8	0	0	0	10	3	0	42			
7:15 AM	1	0	0	0	3	1	5	0	2	8	0	0	0	11	12	0	43			i
7:30 AM	1	0	1	0	7	4	1	0	7	3	0	0	0	17	16	0	57			i
7:45 AM	0	0	2	0	4	1	7	0	8	12	0	0	0	32	15	0	81	223	4	0.688272
8:00 AM	1	0	1	0	3	2	12	0	0	6	0	0	0	24	12	0	61	242	3	0.746914
8:15 AM	1	0	1	0	6	1	11	0	4	2	0	0	0	19	13	0	58	257	1	0.79321
8:30 AM	2	0	0	0	9	1	13	0	6	2	0	0	0	14	5	0	52	252	2	0.777778
8:45 AM	0	0	0	0	12	2	12	0	4	0	0	0	0	13	5	0	48	219	5	0.897541
4:00 PM	3	0	1	0	12	0	3	0	0	9	0	0	0	14	32	0	74			i
4:15 PM	1	0	1	0	14	1	9	0	2	1	0	0	0	19	12	0	60			i
4:30 PM	1	0	0	0	6	3	9	0	3	7	0	0	0	11	30	0	70			
4:45 PM	0	0	0	0	14	0	12	0	1	5	0	0	0	19	32	0	83	287	5	0.864458
5:00 PM	0	0	0	0	24	0	12	0	3	14	0	0	0	21	45	0	119	332	4	0.697479
5:15 PM	0	0	0	0	16	0	12	0	2	6	0	0	0	23	53	0	112	384	3	0.806723
5:30 PM	2	0	2	0	17	2	19	0	1	10	0	0	0	19	34	0	106	420	2	0.882353
5:45 PM	0	0	1	0	18	1	14	0	1	6	0	0	0	17	32	0	90	427	1	0.897059

HSC at Atlas Pear Drive 3/22/2018

Chart																				
Start Time		Eastk	oound			Westl	oound			North	bound			South	bound		15-min	Hourly	Hourly	
Time	Left	Thru	Right	U-Turn	Total	Total	Rank	PHF												
7:00 AM	0	14	0		0	51	7		0	0	0		13	0	5		90			
7:15 AM	0	18	0		0	43	10		0	0	0		16	0	4		91			
7:30 AM	0	21	0		0	60	10		0	0	0		27	0	6		124			
7:45 AM	1	28	0		0	74	6		0	0	0		36	0	3		148	453	5	0.765203
8:00 AM	1	26	0		0	79	13		0	0	0		24	0	6		149	512	2	0.85906
8:15 AM	2	26	0		0	61	2		0	0	0		13	0	6		110	531	1	0.89094
8:30 AM	0	16	0		0	59	9		0	0	0		10	0	7		101	508	3	0.852349
8:45 AM	3	17	0		0	51	5		0	0	0		12	0	8		96	456	4	0.765101
4:00 PM	4	55	0		0	32	6		0	0	0		5	0	1		103			
4:15 PM	3	45	0		0	39	23		0	0	0		6	0	4		120			
4:30 PM	2	51	0		0	40	16		0	0	0		11	0	2		122			
4:45 PM	10	68	0		0	43	25		0	0	0		23	0	4		173	518	4	0.748555
5:00 PM	4	47	0		0	44	23		0	0	0		16	0	5		139	554	3	0.800578
5:15 PM	4	47	0		0	47	24		0	0	0		15	0	1		138	572	2	0.82659
5:30 PM	4	48	0		0	30	25		0	0	0		17	0	2		126	576	1	0.83237
5:45 PM	4	22	0		0	27	18		0	0	0		14	0	5		90	493	5	0.886691

HSC Parkway at South Traditions Drive 3/22/2018

Ctant																				
Start Time		Eastk	ound			West	oound			North	bound			South	bound		15-min	Hourly	Hourly	
Time	Left	Thru	Right	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right	U-Turn	Total	Total	Rank	PHF
7:00 AM	0	4	0	0	4	41	6	0	0	0	0	0	4	0	1	0	60			
7:15 AM	0	5	1	0	6	38	7	0	0	0	3	0	8	0	3	0	71			
7:30 AM	0	2	3	0	11	22	16	0	1	0	0	0	12	0	1	0	68			
7:45 AM	0	2	1	0	3	27	14	0	0	0	1	0	10	0	2	0	60	259	5	0.911972
8:00 AM	0	10	0	0	2	38	17	0	0	0	2	0	13	1	2	0	85	284	4	0.835294
8:15 AM	0	10	1	0	3	45	20	0	1	0	2	0	14	2	3	0	101	314	3	0.777228
8:30 AM	0	12	0	0	2	61	22	0	0	0	0	0	13	0	7	0	117	363	2	0.775641
8:45 AM	0	12	0	0	2	45	23	0	0	0	1	0	14	2	4	0	103	406	1	0.867521
4:00 PM	1	28	0	0	1	16	26	1	1	0	1	0	27	2	1	0	105			
4:15 PM	0	18	0	0	3	8	21	0	0	1	2	0	35	0	2	0	90			
4:30 PM	2	20	0	0	1	12	26	1	1	1	4	0	33	0	1	0	102			
4:45 PM	6	23	4	0	2	16	23	0	7	2	1	0	21	3	2	1	111	408	5	0.918919
5:00 PM	5	37	0	0	0	25	15	1	0	0	0	0	34	0	4	0	121	424	4	0.876033
5:15 PM	0	21	0	0	2	15	39	3	0	0	7	0	23	1	2	0	113	447	2	0.923554
5:30 PM	1	26	0	0	0	12	41	3	1	0	2	0	27	0	1	1	115	460	1	0.950413
5:45 PM	3	15	0	0	0	12	21	0	0	0	9	0	34	1	0	0	95	444	3	0.917355

Turkey Creek Road at HSC Parkway/F&B Road 3/21/2018

. .																				
Start Time		Eastk	ound			Westl	oound			North	bound			South	bound		15-min	Hourly	Hourly	
Tille	Left	Thru	Right	U-Turn	Total	Total	Rank	PHF												
7:00 AM	0	16	9	0	6	41	0	0	11	0	12	0	0	0	0	0	95			
7:15 AM	0	16	16	0	17	48	0	0	7	0	8	0	0	0	0	0	112			
7:30 AM	0	29	26	0	10	54	0	0	8	0	5	0	0	0	0	0	132			
7:45 AM	0	37	22	0	20	64	0	0	7	0	6	0	0	0	0	0	156	495	3	0.793269
8:00 AM	0	23	9	0	13	78	0	0	6	0	7	0	0	0	0	0	136	536	1	0.858974
8:15 AM	0	19	6	1	14	45	0	0	13	0	2	0	0	0	0	0	100	524	2	0.839744
8:30 AM	0	17	13	0	9	48	0	0	10	0	5	0	0	0	0	0	102	494	4	0.791667
8:45 AM	0	15	19	0	7	58	0	0	10	0	1	0	0	0	0	0	110	448	5	0.823529
4:00 PM	0	29	8	0	5	39	0	0	6	0	6	0	0	0	0	0	93			
4:15 PM	0	46	9	0	14	42	0	0	10	0	9	0	0	0	0	0	130			
4:30 PM	0	39	12	0	24	67	0	0	14	0	5	0	0	0	0	0	161			
4:45 PM	0	39	20	0	40	44	0	0	10	0	7	0	0	0	0	0	160	544	5	0.84472
5:00 PM	0	71	23	0	47	64	0	0	15	0	23	0	0	0	0	0	243	694	4	0.713992
5:15 PM	0	66	28	0	69	42	0	0	13	0	6	0	0	0	0	0	224	788	3	0.8107
5:30 PM	0	48	23	0	27	63	0	0	13	0	7	0	0	0	0	0	181	808	2	0.831276
5:45 PM	0	73	27	0	22	65	0	0	18	0	3	0	0	0	0	0	208	856	1	0.880658

Land Use: 220 Multifamily Housing (Low-Rise)

Description

Low-rise multifamily housing includes apartments, townhouses, and condominiums located within the same building with at least three other dwelling units and that have one or two levels (floors). Multifamily housing (mid-rise) (Land Use 221), multifamily housing (high-rise) (Land Use 222), and off-campus student apartment (Land Use 225) are related land uses.

Additional Data

In prior editions of *Trip Generation Manual*, the low-rise multifamily housing sites were further divided into rental and condominium categories. An investigation of vehicle trip data found no clear differences in trip making patterns between the rental and condominium sites within the ITE database. As more data are compiled for future editions, this land use classification can be reinvestigated.

For the three sites for which both the number of residents and the number of occupied dwelling units were available, there were an average of 2.72 residents per occupied dwelling unit.

For the two sites for which the numbers of both total dwelling units and occupied dwelling units were available, an average of 96.2 percent of the total dwelling units were occupied.

This land use included data from a wide variety of units with different sizes, price ranges, locations, and ages. Consequently, there was a wide variation in trips generated within this category. Other factors, such as geographic location and type of adjacent and nearby development, may also have had an effect on the site trip generation.

Time-of-day distribution data for this land use are presented in Appendix A. For the 10 general urban/suburban sites with data, the overall highest vehicle volumes during the AM and PM on a weekday were counted between 7:15 and 8:15 a.m. and 4:45 and 5:45 p.m., respectively. For the one site with Saturday data, the overall highest vehicle volume was counted between 9:45 and 10:45 a.m. For the one site with Sunday data, the overall highest vehicle volume was counted between 11:45 a.m. and 12:45 p.m.

For the one dense multi-use urban site with 24-hour count data, the overall highest vehicle volumes during the AM and PM on a weekday were counted between 7:00 and 8:00 a.m. and 6:15 and 7:15 p.m., respectively.

For the three sites for which data were provided for both occupied dwelling units and residents, there was an average of 2.72 residents per occupied dwelling unit.

The average numbers of person trips per vehicle trip at the five general urban/suburban sites at which both person trip and vehicle trip data were collected were as follows:

- 1.13 during Weekday, Peak Hour of Adjacent Street Traffic, one hour between 7 and 9 a.m.
- 1.21 during Weekday, Peak Hour of Adjacent Street Traffic, one hour between 4 and 6 p.m.



The sites were surveyed in the 1980s, the 1990s, the 2000s, and the 2010s in British Columbia (CAN), California, District of Columbia, Florida, Georgia, Illinois, Indiana, Maine, Maryland, Minnesota, New Jersey, New York, Ontario, Oregon, Pennsylvania, South Dakota, Tennessee, Texas, Utah, Virginia, and Washington.

It is expected that the number of bedrooms and number of residents are likely correlated to the number of trips generated by a residential site. Many of the studies included in this land use did not indicate the total number of bedrooms. To assist in the future analysis of this land use, it is important that this information be collected and included in trip generation data submissions.

Source Numbers

168, 187, 188, 204, 211, 300, 305, 306, 319, 320, 321, 357, 390, 412, 418, 525, 530, 571, 579, 583, 864, 868, 869, 870, 896, 903, 918, 946, 947, 948, 951



Multifamily Housing (Low-Rise)

(220)

Vehicle Trip Ends vs: Dwelling Units

On a: Weekday,

Peak Hour of Adjacent Street Traffic, One Hour Between 7 and 9 a.m.

Setting/Location: General Urban/Suburban

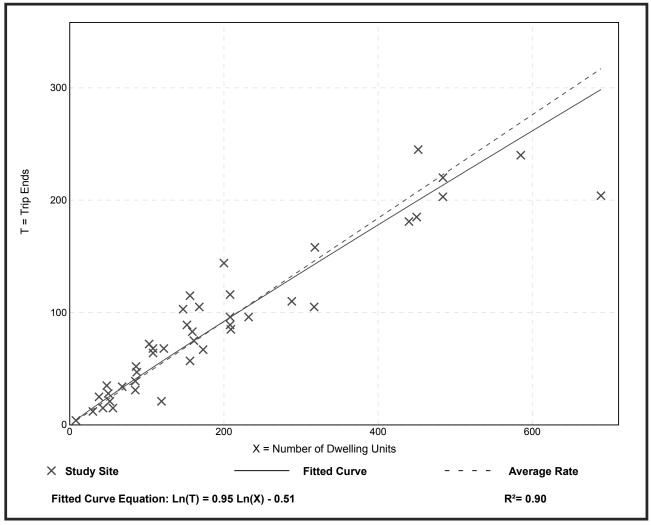
Number of Studies: 42 Avg. Num. of Dwelling Units: 199

Directional Distribution: 23% entering, 77% exiting

Vehicle Trip Generation per Dwelling Unit

Average Rate	Range of Rates	Standard Deviation
0.46	0.18 - 0.74	0.12

Data Plot and Equation



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Multifamily Housing (Low-Rise)

(220)

Vehicle Trip Ends vs: Dwelling Units

On a: Weekday,

Peak Hour of Adjacent Street Traffic, One Hour Between 4 and 6 p.m.

Setting/Location: General Urban/Suburban

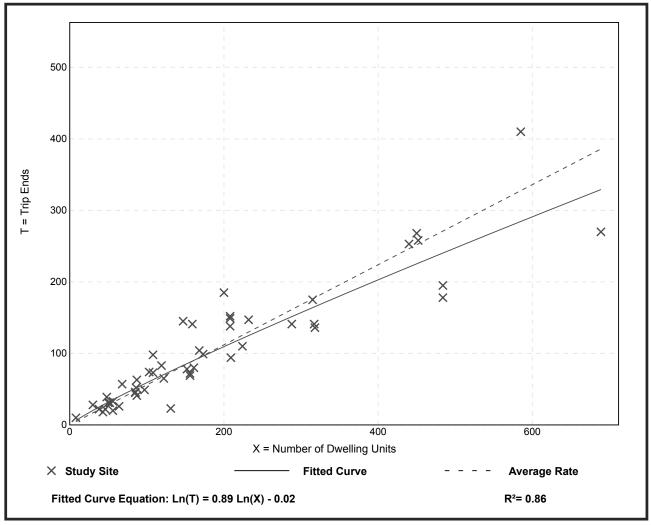
Number of Studies: 50 Avg. Num. of Dwelling Units: 187

Directional Distribution: 63% entering, 37% exiting

Vehicle Trip Generation per Dwelling Unit

Average Rate	Range of Rates	Standard Deviation
0.56	0.18 - 1.25	0.16

Data Plot and Equation



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10.6

EBT

110

2

128

EB

WB

NB

8.5

73%

27%

Stop

96 110

70

112 128

Yes

3.331

0.166

9.4

0.6

0 110

79

60 278

70

0

WB

EB

NB

12.2

0%

Stop

0% 100%

NBLn1 EBLn1 EBLn2 WBLn1

0% 18%

100% 0%

Stop

0 278

92 393

Yes Yes

0.184 0.114 0.503

7.8 12.2

0.4 2.9

2.185 2.642

0.164 0.183 0.113 0.503

Yes

2.89

0.7

EBR WBL WBT NBL NBR

70

2

81 30

EB

2

WB

9.4

82%

0%

Stop

338 79

60 0

26

2

0

0.86

Intersection Intersection Delay, s/veh Intersection LOS

Movement

Lane Configurations Traffic Vol, veh/h Future Vol, veh/h

Peak Hour Factor

Heavy Vehicles, % Mvmt Flow

Number of Lanes

Opposing Approach
Opposing Lanes

Conflicting Approach Left

Conflicting Approach Right

Conflicting Lanes Left

Conflicting Lanes Right HCM Control Delay

Approach

HCM LOS

Vol Left, %

Vol Thru, %

Vol Right, %

Sign Control

Through Vol

Geometry Grp Degree of Util (X)

Cap Service Time

Departure Headway (Hd) Convergence, Y/N

HCM Lane V/C Ratio

HCM Control Delay HCM Lane LOS

HCM 95th-tile Q

06/06/2018

RT Vol Lane Flow Rate

LT Vol

Traffic Vol by Lane

	•	-	*	•	←	*	4	†	1	-	↓	1
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBF
Lane Configurations		4		ሻ	†	7	76	^	7	ሻ	† î>	
Traffic Volume (vph)	39	33	46	68	48	17	170	692	175	179	808	150
Future Volume (vph)	39	33	46	68	48	17	170	692	175	179	808	150
Satd. Flow (prot)	0	1736	0	1770	1863	1583	1770	3539	1583	1770	3458	C
Flt Permitted		0.984		0.950			0.160			0.284		
Satd. Flow (perm)	0	1736	0	1770	1863	1583	298	3539	1583	529	3458	C
Satd. Flow (RTOR)		22				131			203		21	
Lane Group Flow (vph)	0	136	0	79	56	20	198	805	203	208	1114	C
Turn Type	Split	NA		Split	NA	Free	pm+pt	NA	Free	pm+pt	NA	
Protected Phases	3	3		4	4		1	6		5	2	
Permitted Phases						Free	6		Free	2		
Total Split (s)	22.6	22.6		22.6	22.6		23.2	59.3		20.5	56.6	
Total Lost Time (s)		4.5		4.5	4.5		4.5	4.5		4.5	4.5	
Act Effct Green (s)		12.3		10.2	10.2	104.8	67.6	55.8	104.8	66.0	55.0	
Actuated g/C Ratio		0.12		0.10	0.10	1.00	0.65	0.53	1.00	0.63	0.52	
v/c Ratio		0.61		0.46	0.31	0.01	0.55	0.43	0.13	0.45	0.61	
Control Delay		50.4		56.4	51.5	0.0	14.2	17.8	0.2	11.3	21.7	
Queue Delay		0.0		0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Total Delay		50.4		56.4	51.5	0.0	14.2	17.8	0.2	11.3	21.7	
LOS		D		Е	D	Α	В	В	Α	В	С	
Approach Delay		50.4			47.4			14.3			20.0	
Approach LOS		D			D			В			С	
Queue Length 50th (ft)		74		52	36	0	47	173	0	49	270	
Queue Length 95th (ft)		140		104	79	0	93	270	0	97	427	
Internal Link Dist (ft)		2234			595			1612			403	
Turn Bay Length (ft)				125		300	200		500	200		
Base Capacity (vph)		322		310	327	1583	469	1884	1583	543	1825	
Starvation Cap Reductn		0		0	0	0	0	0	0	0	0	
Spillback Cap Reductn		0		0	0	0	0	0	0	0	0	
Storage Cap Reductn		0		0	0	0	0	0	0	0	0	
Reduced v/c Ratio		0.42		0.25	0.17	0.01	0.42	0.43	0.13	0.38	0.61	
Intersection Summary												
Cycle Length: 125	0											
Actuated Cycle Length: 104												
Control Type: Actuated-Unc	oordinated											
Maximum v/c Ratio: 0.61												

intersection summary		
Cycle Length: 125		
Actuated Cycle Length: 104.8		
Control Type: Actuated-Uncoordinated		
Maximum v/c Ratio: 0.61		
Intersection Signal Delay: 20.5	Intersection LOS: C	
Intersection Capacity Utilization 61.2%	ICU Level of Service B	
Analysis Period (min) 15		

Splits and Phases: 1: FM 2818 & F and B Road/HSC Parkway



BBI		Synchro 10 Report

BBI 06/06/2018 Synchro 10 Report

Intersection Int Delay, s/veh	3											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	ሻ	∱ ⊅		ሻ	∱ }			4			4	
Traffic Vol, veh/h	4	112	0	0	350	31	0	0	0	100	0	21
Future Vol, veh/h	4	112	0	0	350	31	0	0	0	100	0	21
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	100	-	-	150	-	-	-	-	-	-	-	-
Veh in Median Storage,		0		-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	89	89	89	89	89	89	89	89	89	89	89	89
Heavy Vehicles, %	2	2	2	2	2	2	2	2	2	2	2	2
Mvmt Flow	4	126	0	0	393	35	0	0	0	112	0	24
Major/Minor N	/lajor1		N	/lajor2		N	/linor1		Λ	/linor2		
Conflicting Flow All	428	0	0	126	0	0	331	562	63	482	545	214
Stage 1	-		-	-	-	-	134	134	-	411	411	
Stage 2	-	-	-		-	-	197	428	-	71	134	
Critical Hdwy	4.14	-	-	4.14	-	-	7.54	6.54	6.94	7.54	6.54	6.94
Critical Hdwy Stg 1	-	-	-	-	-	-	6.54	5.54	-	6.54	5.54	-
Critical Hdwy Stg 2	-	-	-	-	-	-	6.54	5.54	-	6.54	5.54	
Follow-up Hdwy	2.22	-	-	2.22	-	-	3.52	4.02	3.32	3.52	4.02	3.32
Pot Cap-1 Maneuver	1128	-	-	1458	-	-	599	434	988	467	444	791
Stage 1	-	-	-	-	-	-	855	785	-	589	593	-
Stage 2	-	-	-	-	-	-	786	583	-	931	785	
Platoon blocked, %		-	-		-	-						
Mov Cap-1 Maneuver	1128	-		1458	-	-	580	432	988	466	442	791
Mov Cap-2 Maneuver	-	-	-	-	-	-	580	432	-	466	442	-
Stage 1	-	-	-	-	-	-	852	782		587	593	-
Stage 2	-	-	-	-	-	-	763	583	-	928	782	-
Approach	EB			WB	_		NB			SB		
HCM Control Delay, s	0.3			0			0			14.8		
HCM LOS	0.0			- 3			A			В		
Minor Lane/Major Mvm	1 1	VBLn1	EBL	EBT	EBR	WBL	WBT	WBR :	SRI n1			
Capacity (veh/h)		VDEIII	1128	-	LDIN	1458	-	WDIX.	502			
HCM Lane V/C Ratio			0.004			1430			0.271			
HCM Control Delay (s)		0	8.2			0			14.8			
HCM Lane LOS		A	0.2 A			A			14.0 B			
HCM 95th %tile Q(veh)		А	0			0			1.1			

Intersection												
Intersection Delay, s/veh	8.7											
Intersection LOS	A											
microsolion 200	,,											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBF
Lane Configurations	*	† 1>		ሻ	ħβ			4			4	
Traffic Vol, veh/h	0	44	9	85	189	82	2	0	16	54	5	10
Future Vol. veh/h	0	44	9	85	189	82	2	0	16	54	5	10
Peak Hour Factor	0.87	0.87	0.87	0.87	0.87	0.87	0.87	0.87	0.87	0.87	0.87	0.8
Heavy Vehicles, %	2	2	2	2	2	2	2	2	2	2	2	
Mvmt Flow	0	51	10	98	217	94	2	0	18	62	6	18
Number of Lanes	1	2	0	1	2	0	0	1	0	0	1	(
Approach	EB			WB			NB			SB		
Opposing Approach	WB			EB			SB			NB		
Opposing Lanes	3			3			1			1		
Conflicting Approach Left	SB			NB			EB			WB		
Conflicting Lanes Left	1			1			3			3		
Conflicting Approach Right	NB			SB			WB			EB		
Conflicting Lanes Right	1			1			3			3		
HCM Control Delay	8.1			8.7			8			9.5		
HCM LOS	Α			Α			Α			Α		
Lane		NBLn1	EBLn1	EBLn2	EBLn3	WBLn1	WBLn2	WBLn3	SBLn1			
Vol Left, %		11%	0%	0%	0%	100%	0%	0%	72%			
Vol Thru, %		0%	100%	100%	62%	0%	100%	43%	7%			
Vol Right, %		89%	0%	0%	38%	0%	0%	57%	21%			
Sign Control		Stop	Stop	Stop	Stop	Stop	Stop	Stop	Stop			
Traffic Vol by Lane		18	0	29	24	85	126	145	75			
LT Vol		2	0	0	0	85	0	0	54			
Through Vol		0	0	29	15	0	126	63	5			
RT Vol		16	0	0	9	0	0	82	16			
Lane Flow Rate		21	0	34	27	98	145	167	86			
Geometry Grp		7	7	7	7	7	7	7	7			
Degree of Util (X)		0.029	0	0.049	0.037	0.147	0.197	0.208	0.139			
Departure Headway (Hd)		5.107	5.214	5.214	4.946	5.4	4.898	4.501	5.791			
Convergence, Y/N		Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes			
Cap		700	0	686	723	665	734	798	620			
Service Time		2.847	2.949	2.949	2.681	3.124	2.622	2.224	3.524			
HCM Lane V/C Ratio		0.03	0	0.05	0.037	0.147	0.198	0.209	0.139			
HCM Control Delay		8	7.9	8.2	7.9	9.1	8.8	8.4	9.5			
HCM Lane LOS HCM 95th-tile Q		A 0.1	N 0	A 0.2	A 0.1	A 0.5	A 0.7	A 0.8	A 0.5			

Intersection Intersection Delay, s/veh	8.6											
Intersection LOS	Α.											
intersection LOS	А											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		44						*	7	Ť	^	
Traffic Vol, veh/h	37	4	6	0	0	0	0	4	14	105	26	0
Future Vol, veh/h	37	4	6	0	0	0	0	4	14	105	26	0
Peak Hour Factor	0.75	0.75	0.75	0.75	0.75	0.75	0.75	0.75	0.75	0.75	0.75	0.75
Heavy Vehicles, %	2	2	2	2	2	2	2	2	2	2	2	2
Mvmt Flow	49	5	8	0	0	0	0	5	19	140	35	0
Number of Lanes	0	1	0	0	0	0	0	1	1	1	2	0
Approach	EB							NB		SB		
Opposing Approach								SB		NB		
Opposing Lanes	0							3		2		
Conflicting Approach Left	SB							EB				
Conflicting Lanes Left	3							1		0		
Conflicting Approach Right	NB									EB		
Conflicting Lanes Right	2							0		1		
HCM Control Delay	8.6							7.3		8.8		
HCM LOS	Α							Α		Α		
Lane		NBLn1	NBLn2	EBLn1	SBLn1	SBLn2	SBLn3					
Vol Left, %		0%	0%	79%	100%	0%	0%					
Vol Thru, %		100%	0%	9%	0%	100%	100%					
Vol Right, %		0%	100%	13%	0%	0%	0%					
Sign Control		Stop	Stop	Stop	Stop	Stop	Stop					
Traffic Vol by Lane		4	14	47	105	13	13					
LT Vol		0	0	37	105	0	0					
Through Vol		4	0	4	0	13	13					
RT Vol		0	14	6	0	0	0					
Lane Flow Rate		5	19	63	140	17	17					
Geometry Grp		8	8	7	7	7	7					
Degree of Util (X)		0.008	0.023	0.093	0.201	0.022	0.014					
Departure Headway (Hd)		5.084	4.382	5.317	5.159	4.658	2.922					
Convergence, Y/N		Yes	Yes	Yes	Yes	Yes	Yes					
Cap		707	821	677	689	760	1200					
Service Time		2.791	2.089	3.021	2.94	2.438	0.701					
HCM Lane V/C Ratio		0.007	0.023	0.093	0.203	0.022	0.014					
HCM Control Delay		7.8	7.2	8.6	9.3	7.5	5.7					
		Α	Α	Α	Α	Α	Α					
HCM Lane LOS		A	A	A	A	A	A					

0.1

0.3 0.7

Intersection												
Intersection Delay, s/veh	8.1											
Intersection LOS	Α											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4		ሻ	†			4	
Traffic Vol, veh/h	3	0	5	20	8	73	19	23	0	0	98	56
Future Vol, veh/h	3	0	5	20	8	73	19	23	0	0	98	56
Peak Hour Factor	0.79	0.79	0.79	0.79	0.79	0.79	0.79	0.79	0.79	0.79	0.79	0.79
Heavy Vehicles, %	2	2	2	2	2	2	2	2	2	2	2	2
Mvmt Flow	4	0	6	25	10	92	24	29	0	0	124	71
Number of Lanes	0	1	0	0	1	0	1	1	0	0	1	C
Approach	EB			WB			NB				SB	
Opposing Approach	WB			EB			SB				NB	
Opposing Lanes	1			1			1				2	
Conflicting Approach Left	SB			NB			EB				WB	
Conflicting Lanes Left	1			2			1				1	
Conflicting Approach Right	NB			SB			WB				EB	
Conflicting Lanes Right	2			1			1				1	
HCM Control Delay	7.4			7.8			8.1				8.4	
HCM LOS	Α			Α			А				Α	
Lane		NBLn1	NBLn2	EBLn1	WBLn1	SBLn1						
Vol Left, %		100%	0%	38%	20%	0%						
Vol Thru, %		0%	100%	0%	8%	64%						
Vol Right, %		0%	0%	62%	72%	36%						
Sign Control		Stop	Stop	Stop	Stop	Stop						
Traffic Vol by Lane		19	23	8	101	154						
LT Vol		19	0	3	20	0						
Through Vol		0	23	0	8	98						
RT Vol		0	0	5	73	56						
Lane Flow Rate		24	29	10	128	195						
Geometry Grp		7	7	2	2	5						
Degree of Util (X)		0.037	0.04	0.012	0.146	0.222						
Departure Headway (Hd)		5.492	4.99	4.334	4.113	4.098						
Convergence, Y/N		Yes	Yes	Yes	Yes	Yes						
Сар		656	722	829	877	862						
Service Time		3.192	2.69	2.341	2.115	2.193						
HOME VIO D-#-		0.007	0.04	0.040	0.447	0.007						

0

7.4 7.8 8.4

0.5 0.8

8.4 7.9

0.1 0.1

HCM 95th-tile Q

HCM Lane V/C Ratio

HCM Control Delay HCM Lane LOS

HCM 95th-tile Q

Existing (2018)

HCM Lane V/C Ratio

HCM Control Delay HCM Lane LOS

HCM 95th-tile Q

	۶	→	\rightarrow	•	←	*	1	†	1	-	↓	4
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4		7	^	7	7	44	7	7	ħβ	
Traffic Volume (vph)	121	97	121	167	68	104	167	1144	121	45	788	140
Future Volume (vph)	121	97	121	167	68	104	167	1144	121	45	788	140
Satd. Flow (prot)	0	1741	0	1770	1863	1583	1770	3539	1583	1770	3458	0
Flt Permitted		0.982		0.950			0.100			0.092		
Satd. Flow (perm)	0	1741	0	1770	1863	1583	186	3539	1583	171	3458	0
Satd. Flow (RTOR)		20				236			236		17	
Lane Group Flow (vph)	0	369	0	182	74	113	182	1243	132	49	1009	0
Turn Type	Split	NA		Split	NA	Free	pm+pt	NA	Free	pm+pt	NA	
Protected Phases	3	3		4	4		1	6		5	2	
Permitted Phases						Free	6		Free	2		
Total Split (s)	34.0	34.0		23.0	23.0		18.0	55.0		13.0	50.0	
Total Lost Time (s)		7.0		7.0	7.0		8.0	8.0		8.0	8.0	
Act Effct Green (s)		26.5		15.2	15.2	123.7	57.6	49.7	123.7	47.0	42.0	
Actuated g/C Ratio		0.21		0.12	0.12	1.00	0.47	0.40	1.00	0.38	0.34	
v/c Ratio		0.95		0.84	0.32	0.07	0.85	0.87	0.08	0.38	0.85	
Control Delay		81.1		83.7	53.8	0.1	57.9	43.4	0.1	27.1	45.7	
Queue Delay		0.0		0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Total Delay		81.1		83.7	53.8	0.1	57.9	43.4	0.1	27.1	45.7	
LOS		F		F	D	Α	Е	D	Α	С	D	
Approach Delay		81.1			52.1			41.4			44.9	
Approach LOS		F			D			D			D	
Queue Length 50th (ft)		282		145	55	0	86	504	0	21	394	
Queue Length 95th (ft)		#475		#265	104	0	#219	#649	0	43	483	
Internal Link Dist (ft)		2234			595			1612			403	
Turn Bay Length (ft)				125		300	200		500	200		
Base Capacity (vph)		395		228	241	1583	214	1422	1583	129	1185	
Starvation Cap Reductn		0		0	0	0	0	0	0	0	0	
Spillback Cap Reductn		0		0	0	0	0	0	0	0	0	
Storage Cap Reductn		0		0	0	0	0	0	0	0	0	
Reduced v/c Ratio		0.93		0.80	0.31	0.07	0.85	0.87	0.08	0.38	0.85	
Intersection Summary												
Cycle Length: 125												
Actuated Cycle Length: 123.7												
Control Type: Actuated-Uncoo	rdinated											
Maximum v/c Ratio: 0.95												
Intersection Signal Delay: 48.1					tersection							
Intersection Capacity Utilizatio	n 80.8%			IC	U Level	of Service	e D					
Analysis Period (min) 15												
# 95th percentile volume exc	ceeds ca	pacity, qu	eue may	be longer								
Queue shown is maximum	after two	cycles.										

Splits and Phases: 1: FM 2818 & F and B Road/HSC Parkway **♣**ø₃ **7**∅4

Synchro 10 Report 06/06/2018

							•
Intersection							
Intersection Delay, s/veh	13.5						
Intersection LOS	В						
Movement	EBT	EBR	WBL	WBT	NBL	NBR	
Lane Configurations	†	7		લ	¥		_
Traffic Vol, veh/h	288	139	165	240	67	39	
Future Vol, veh/h	288	139	165	240	67	39	
Peak Hour Factor	0.88	0.88	0.88	0.88	0.88	0.88	
Heavy Vehicles, %	2	2	2	2	2	2	
Mvmt Flow	327	158	188	273	76	44	
Number of Lanes	1	1	0	1	1	0	
Approach	EB		WB		NB		
Opposing Approach	WB		EB				
Opposing Lanes	1		2		0		
Conflicting Approach Left			NB		EB		
Conflicting Lanes Left	0		1		2		
Conflicting Approach Right	NB				WB		
Conflicting Lanes Right	1		0		1		
HCM Control Delay	11.5		16.4		10.4		
HCM LOS	В		С		В		
Lane		NBLn1	EBLn1	EBLn2	WBLn1		
Vol Left, %		63%	0%	0%	41%		
Vol Thru, %		0%	100%	0%	59%		
Vol Right, %		37%	0%	100%	0%		
Sign Control		Stop	Stop	Stop	Stop		
Traffic Vol by Lane		106	288	139	405		
LT Vol		67	0	0	165		
Through Vol		0	288	0	240		
RT Vol		39	0	139	0		
Lane Flow Rate		120	327	158	460		
Geometry Grp		2	7	7	5		
Degree of Util (X)		0.199	0.48	0.201	0.635		
Departure Headway (Hd)		5.956	5.284	4.577	4.969		
Convergence, Y/N		Yes	Yes	Yes	Yes		
Сар		606	677	776	723		
Service Time		3.956	3.063	2.356	3.043		

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0.483 0.204 0.636

0.7 4.6

8.5 16.4

0.198

10.4 12.9

0.7

2.6

Intersection Int Delay, s/veh	2.1											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		ħβ			ħβ			4			4	
Traffic Vol, veh/h	22	278	0	0	178	97	0	0	0	71	0	12
Future Vol, veh/h	22	278	0	0	178	97	0	0	0	71	0	12
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None		-	None	-	-	None
Storage Length	100	-	-	150	-	-	-	-	-	-	-	-
Veh in Median Storage,		0	-	-	0	-		0	-	-	0	
Grade, %	-	0	-	-	0	-	-	0		-	0	-
Peak Hour Factor	83	83	83	83	83	83	83	83	83	83	83	83
Heavy Vehicles, %	2	2	2	2	2	2	2	2	2	2	2	2
Mvmt Flow	27	335	0	0	214	117	0	0	0	86	0	14
Major/Minor M	lajor1		N	/lajor2		1	Vinor1		N_	/linor2		
Conflicting Flow All	331	0	0	335	0	0	496	720	168	495	662	166
Stage 1	-	-	-	-		-	389	389	-	273	273	
Stage 2	-	-	-	-	-	-	107	331	-	222	389	-
Critical Hdwy	4.14	-	-	4.14	-	-	7.54	6.54	6.94	7.54	6.54	6.94
Critical Hdwy Stg 1	-	-	-	-	-	-	6.54	5.54	-	6.54	5.54	-
Critical Hdwy Stg 2	-	-	-	-	-	-	6.54	5.54	-	6.54	5.54	
Follow-up Hdwy	2.22	-	-	2.22	-	-	3.52	4.02	3.32	3.52	4.02	3.32
Pot Cap-1 Maneuver	1225	-	-	1221	-	-	457	352	847	457	381	849
Stage 1	-	-	-	-	-	-	606	607	-	710	683	-
Stage 2	-	-	-	-	-	-	887	644	-	760	607	-
Platoon blocked, %		-	-		-	-						
	1225	-	-	1221	-	-	441	344	847	449	373	849
Mov Cap-2 Maneuver	-	-	-	-	-	-	441	344	-	449	373	-
Stage 1	-	-	-	-	-	-	593	594	-	694	683	-
Stage 2	-	-	-	-	-	-	872	644	-	743	594	-
Approach	EB			WB			NB			SB		
HCM Control Delay, s	0.6			0			0			14.4		
HCM LOS				-			A			В		
Minor Lane/Major Mvmt		NBLn1	EBL	EBT	EBR	WBL	WBT	WBR :	CDI n1			
	ľ		1225			1221		WBR:	482			
Capacity (veh/h)		-		-	-		-					
HCM Lane V/C Ratio HCM Control Delay (s)		-	0.022	-	-	-	-	-	0.207			
		0	8	-	-	0		-	14.4			
						Α.			D			
HCM Lane LOS HCM 95th %tile Q(veh)		A	A 0.1			A 0			B 0.8			

Intersection												
Intersection Delay, s/veh	9											
Intersection LOS	Α											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBF
Lane Configurations	*	† \$		ች	† 1>			44			4	
Traffic Vol, veh/h	12	107	6	18	68	118	16	2	78	105	4	(
Future Vol. veh/h	12	107	6	18	68	118	16	2	78	105	4	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.93
Heavy Vehicles, %	2	2	2	2	2	2	2	2	2	2	2	
Mvmt Flow	13	116	7	20	74	128	17	2	85	114	4	10
Number of Lanes	1	2	0	1	2	0	0	1	0	0	1	(
Approach	EB			WB			NB			SB		
Opposing Approach	WB			EB			SB			NB		
Opposing Lanes	3			3			1			1		
Conflicting Approach Left	SB			NB			EB			WB		
Conflicting Lanes Left	1			1			3			3		
Conflicting Approach Right	NB			SB			WB			EB		
Conflicting Lanes Right	1			1			3			3		
HCM Control Delay	8.7			8.7			8.7			10.2		
HCM LOS	۸									D		
I ICIVI EUS	Α			Α			Α			В		
HCW E03	А			А			А			В		
Lane	А	NBLn1	EBLn1	EBLn2	EBLn3	WBLn1	WBLn2	WBLn3	SBLn1	В		
Lane	А	NBLn1 17%	EBLn1 100%		EBLn3	WBLn1 100%		WBLn3	SBLn1	В		
Lane Vol Left, %	A			EBLn2			WBLn2			В		
Lane	A	17%	100%	EBLn2	0%	100%	WBLn2	0%	89%	В		
Lane Vol Left, % Vol Thru, %	A	17% 2%	100% 0%	EBLn2 0% 100%	0% 86%	100% 0%	WBLn2 0% 100%	0% 16%	89% 3%	В		
Lane Vol Left, % Vol Thru, % Vol Right, %	A	17% 2% 81%	100% 0% 0%	EBLn2 0% 100% 0%	0% 86% 14%	100% 0% 0%	WBLn2 0% 100% 0%	0% 16% 84%	89% 3% 8%	В		
Lane Vol Left, % Vol Thru, % Vol Right, % Sign Control	A	17% 2% 81% Stop	100% 0% 0% Stop	EBLn2 0% 100% 0% Stop	0% 86% 14% Stop	100% 0% 0% Stop	WBLn2 0% 100% 0% Stop	0% 16% 84% Stop	89% 3% 8% Stop	В		
Lane Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane	A	17% 2% 81% Stop 96 16 2	100% 0% 0% Stop 12	EBLn2 0% 100% 0% Stop 71	0% 86% 14% Stop 42	100% 0% 0% Stop 18	WBLn2 0% 100% 0% Stop 45	0% 16% 84% Stop 141	89% 3% 8% Stop 118	В		
Lane Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol	A	17% 2% 81% Stop 96 16 2 78	100% 0% 0% Stop 12 12 0	EBLn2 0% 100% 0% Stop 71 0 71	0% 86% 14% Stop 42 0 36	100% 0% 0% Stop 18 18 0	WBLn2 0% 100% 0% Stop 45 0 45	0% 16% 84% Stop 141 0 23 118	89% 3% 8% Stop 118 105 4	В		
Lane Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol	A	17% 2% 81% Stop 96 16 2 78	100% 0% 0% Stop 12 12 0 0	EBLn2 0% 100% 0% Stop 71 0 71 0 78	0% 86% 14% Stop 42 0 36 6	100% 0% 0% Stop 18 18 0 0	WBLn2 0% 100% 0% Stop 45 0 45 0 45	0% 16% 84% Stop 141 0 23 118 153	89% 3% 8% Stop 118 105 4	В		
Lane Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RTANE LT Vol LTANE LT Vol Control REPORT	A	17% 2% 81% Stop 96 16 2 78	100% 0% 0% Stop 12 12 0 0	EBLn2 0% 100% 0% Stop 71 0 71	0% 86% 14% Stop 42 0 36 6 45	100% 0% 0% Stop 18 18 0 0	WBLn2 0% 100% 0% Stop 45 0 45 0 47	0% 16% 84% Stop 141 0 23 118 153	89% 3% 8% Stop 118 105 4 9 128	В		
Lane Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate Geometry Grp Degree of Util (X)	A	17% 2% 81% Stop 96 16 2 78 104 7	100% 0% 0% Stop 12 12 0 0 13 7	EBLn2 0% 100% 0% Stop 71 0 71 77 0.116	0% 86% 14% Stop 42 0 36 6 45 7	100% 0% 0% Stop 18 18 0 0 20 7	WBLn2 0% 100% 0% Stop 45 0 45 0 47 0.073	0% 16% 84% Stop 141 0 23 118 153 7 0.201	89% 3% 8% Stop 118 105 4 9 128 7 0.208	В		
Lane Vol Left, % Vol Trhru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate Geometry Grp Degree of Util (X) Departure Headway (Hd)	A	17% 2% 81% Stop 96 16 2 78 104	100% 0% 0% Stop 12 12 0 0 13 7 0.021 5.913	EBLn2 0% 100% 0% Stop 71 0 71 71 77 77	0% 86% 14% Stop 42 0 36 6 45 7 0.067 5.306	100% 0% 0% Stop 18 18 0 0	WBLn2 0% 100% 0% Stop 45 0 45 0 47	0% 16% 84% Stop 141 0 23 118 153 7 0.201 4.742	89% 3% 8% Stop 118 105 4 9 128 7 0.208 5.846	В		
Lane Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate Geometry Grp Degree of Util (X) Departure Headway (Hd) Convergence, Y/N	A	17% 2% 81% Stop 96 16 2 78 104 7 0.146 5.023 Yes	100% 0% Stop 12 12 0 0 13 7 0.021 5.913 Yes	EBLn2 0% 100% 0% Stop 71 0 71 0 78 7 0.116 5.408 Yes	0% 86% 14% Stop 42 0 36 6 45 7 0.067 5.306 Yes	100% 0% 0% Stop 18 18 0 0 20 7 0.032 5.839 Yes	WBLn2 0% 100% 0% Stop 45 0 45 0 45 0 7 0.073 5.334 Yes	0% 16% 84% Stop 141 0 23 118 153 7 0.201 4.742 Yes	89% 3% 8% Stop 118 105 4 9 128 7 0.208 5.846 Yes	В		
Lane Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate Geometry Grp Degree of Util (X) Departure Headway (Hd) Cap	A	17% 2% 81% Stop 96 16 2 78 104 7 0.146 5.023 Yes 710	100% 0% 0% Stop 12 12 0 0 13 7 0.021 5.913 Yes 603	EBLn2 0% 100% Stop 71 0 71 0 78 7 0.116 5.408 Yes 660	0% 86% 14% Stop 42 0 36 6 45 7 0.067 5.306 Yes 672	100% 0% 0% Stop 18 18 0 20 7 0.032 5.839 Yes 611	WBLn2 0% 100% 5top 45 0 45 0 0 5top 7 0.073 5.334 Yes 669	0% 16% 84% Stop 141 0 23 118 153 7 0.201 4.742 Yes 754	89% 3% 8% Stop 118 105 4 9 128 7 0.208 5.846 Yes 612	В		
Lane Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate Geometry Grp Degree of Util (X) Departure Headway (Hd) Convergence, Y/N Cap Service Time	A	17% 2% 81% Stop 96 16 2 78 104 7 0.146 5.023 Yes 710 2.785	100% 0% 0% Stop 12 0 0 13 7 0.021 5.913 Yes 603 3.672	EBLn2 0% 100% Stop 71 0 71 0 78 7 0.116 5.408 Yes 660 3.167	0% 86% 14% Stop 42 0 36 6 45 7 0.067 5.306 Yes 672 3.065	100% 0% 0% Stop 18 18 0 20 7 0.032 5.839 Yes 611 3.592	WBLn2 0% 100% 5top 45 0 45 0 0 7 0.073 5.334 Yes 669 3.087	0% 16% 84% Stop 141 0 23 118 153 7 0.201 4.742 Yes 754 2.493	89% 3% 8% Stop 118 105 4 9 128 7 0.208 5.846 Yes 612 3.606	В		
Lane Vol Left, % Vol Trhru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate Geometry Grp Degree of Util (X) Departure Headway (Hd) Convergence, Y/N Cap Service Time HCM Lane V/C Ratio	A	17% 2% 81% Stop 96 16 2 78 104 7 0.146 5.023 Yes 710 2.785 0.146	100% 0% 0% Stop 12 12 0 0 13 7 0.021 5.913 Yes 603 3.672 0.022	EBLn2 0% 100% 0% Stop 71 0 71 0 78 7 0.116 5.408 Yes 6660 3.167 0.118	0% 86% 14% Stop 42 0 36 6 45 7 0.067 5.306 Yes 672 3.065 0.067	100% 0% 0% Stop 18 18 0 20 7 0.032 5.839 Yes 611 3.592 0.033	WBLn2 0% 100% 0% Stop 45 0 45 0 47 7 0.073 5.334 Yes 669 3.087 0.073	0% 16% 84% Stop 141 0 23 118 153 7 0.201 4.742 Yes 754 2.493 0.203	89% 3% 8% Stop 118 105 4 9 128 7 0.208 5.846 Yes 612 3.606 0.209	В		
Lane Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate Geometry Grp Degree of Util (X) Departure Headway (Hd) Convergence, Y/N Cap Service Time HCM Lane V/C Ratio HCM Control Delay	A	17% 2% 81% Stop 96 16 2 78 104 7 0.146 5.023 Yes 710 2.785 0.146 8.7	100% 0% 0% Stop 12 12 0 0 13 7 0.021 5.913 Yes 603 3.672 0.022 8.8	EBLn2 0% 100% 0% Stop 71 0 71 0 78 7 0.116 5.408 Yes 660 3.167 0.118	0% 86% 14% Stop 42 0 36 6 45 7 0.067 5.306 Yes 672 3.065 0.067 8.4	100% 0% 0% Stop 18 18 0 20 7 0.032 5.839 Yes 611 3.592 0.033 8.8	WBLn2 0% 100% 0% Stop 45 0 45 0 47 0.073 5.334 Yes 669 3.087 0.073 8.5	0% 16% 84% Stop 141 0 23 118 153 7 0.201 4.742 Yes 754 2.493 0.203 8.7	89% 3% 8% Stop 118 105 4 9 128 7 0.208 5.846 Yes 612 3.606 0.209 10.2	В		
Lane Vol Left, % Vol Trhru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate Geometry Grp Degree of Util (X) Departure Headway (Hd) Convergence, Y/N Cap Service Time HCM Lane V/C Ratio	A	17% 2% 81% Stop 96 16 2 78 104 7 0.146 5.023 Yes 710 2.785 0.146	100% 0% 0% Stop 12 12 0 0 13 7 0.021 5.913 Yes 603 3.672 0.022	EBLn2 0% 100% 0% Stop 71 0 71 0 78 7 0.116 5.408 Yes 6660 3.167 0.118	0% 86% 14% Stop 42 0 36 6 45 7 0.067 5.306 Yes 672 3.065 0.067	100% 0% 0% Stop 18 18 0 20 7 0.032 5.839 Yes 611 3.592 0.033	WBLn2 0% 100% 0% Stop 45 0 45 0 47 7 0.073 5.334 Yes 669 3.087 0.073	0% 16% 84% Stop 141 0 23 118 153 7 0.201 4.742 Yes 754 2.493 0.203	89% 3% 8% Stop 118 105 4 9 128 7 0.208 5.846 Yes 612 3.606 0.209	В		

Intersection											_	
Intersection Delay, s/veh	8.5											
Intersection LOS	Α.5											
IIILEISECLIOII LOS	А											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4						*	7	ħ	^	
Traffic Vol, veh/h	34	6	4	0	0	0	0	7	40	138	59	0
Future Vol, veh/h	34	6	4	0	0	0	0	7	40	138	59	(
Peak Hour Factor	0.82	0.82	0.82	0.82	0.82	0.82	0.82	0.82	0.82	0.82	0.82	0.82
Heavy Vehicles, %	2	2	2	2	2	2	2	2	2	2	2	2
Mvmt Flow	41	7	5	0	0	0	0	9	49	168	72	0
Number of Lanes	0	1	0	0	0	0	0	1	1	1	2	C
Approach	EB							NB		SB		
Opposing Approach								SB		NB		
Opposing Lanes	0							3		2		
Conflicting Approach Left	SB							EB				
Conflicting Lanes Left	3							1		0		
Conflicting Approach Right	NB									EB		
Conflicting Lanes Right	2							0		1		
HCM Control Delay	8.7							7.6		8.7		
HCM LOS	Α							Α		Α		
Lane		NBLn1	NBLn2	EBLn1	SBLn1	SBLn2	SBLn3					
Vol Left, %		0%	0%	77%	100%	0%	0%					
Vol Thru, %		100%	0%	14%	0%	100%	100%					
Vol Right, %		0%	100%	9%	0%	0%	0%					
Sign Control		Stop	Stop	Stop	Stop	Stop	Stop					
Traffic Vol by Lane		7	40	44	138	30	30					
LT Vol		0	0	34	138	0	0					
Through Vol		7	0	6	0	30	30					
RT Vol		0	40	4	0	0	0					
Lane Flow Rate		9	49	54	168	36	36					
Geometry Grp		8	8	7	7	7	7					
Degree of Util (X)		0.012	0.061	0.083	0.241	0.047	0.029					
Departure Headway (Hd)		5.17	4.468	5.539	5.159	4.659	2.922					
Convergence, Y/N		Yes	Yes	Yes	Yes	Yes	Yes					
Cap		695	805	650	688	759	1199					
			0 477	3.244	2.944	2.443	0.705					
Service Time		2.879	2.177									
		0.013	0.061	0.083	0.244	0.047	0.03					
Service Time												

0.2

0.3

0.9

0.1

Intersection												
Intersection Delay, s/veh	9.1											
Intersection LOS	Α											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBF
Lane Configurations		4			4		ሻ	^			44	
Traffic Vol, veh/h	2	0	3	75	3	65	7	36	0	0	118	164
Future Vol, veh/h	2	0	3	75	3	65	7	36	0	0	118	164
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles, %	2	2	2	2	2	2	2	2	2	2	2	2
Mvmt Flow	2	0	3	83	3	72	8	40	0	0	131	182
Number of Lanes	0	1	0	0	1	0	1	1	0	0	1	(
Approach	EB			WB			NB				SB	
Opposing Approach	WB			EB			SB				NB	
Opposing Lanes	1			1			1				2	
Conflicting Approach Left	SB			NB			EB				WB	
Conflicting Lanes Left	1			2			1				1	
Conflicting Approach Right	NB			SB			WB				EB	
Conflicting Lanes Right	2			1			1				1	
HCM Control Delay	7.7			8.7			8.2				9.5	
HCM LOS	Α			Α			А				Α	
Lane		NBLn1	NBLn2	EBLn1	WBLn1	SBLn1						
Vol Left, %		100%	0%	40%	52%	0%						
Vol Thru, %		0%	100%	0%	2%	42%						
Vol Right, %		0%	0%	60%	45%	58%						
Sign Control		Stop	Stop	Stop	Stop	Stop						
Traffic Vol by Lane		7	36	5	143	282						
LT Vol		7	0	2	75	0						
Through Vol		0	36	0	3	118						
RT Vol		0	0	3	65	164						
Lane Flow Rate		8	40	6	159	313						
Geometry Grp		7	7	2	2	5						
Degree of Util (X)		0.012	0.057	0.007	0.201	0.36						
Departure Headway (Hd)		5.672	5.169	4.645	4.563	4.141						
Convergence, Y/N		Yes	Yes	Yes	Yes	Yes						
Cap Camina Time		632	694	770	788	871						
Service Time		3.396	2.892	2.676	2.586	2.156						
HCM Cantral Dalan		0.013	0.058	0.008	0.202	0.359						
HCM Control Delay		8.5	8.2	7.7	8.7	9.5						
HCM Lane LOS HCM 95th-tile Q		A 0	0.2	Α	Α	Α						
				0	0.7	1.6						

HCM 95th-tile Q

HCM 95th-tile Q

	ၨ	→	•	•	+	4	1	†	~	-	↓	4
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4		Ĭ	†	7	7	^	7	7	↑ î>	
Traffic Volume (vph)	45	38	53	79	56	20	197	801	203	207	935	174
Future Volume (vph)	45	38	53	79	56	20	197	801	203	207	935	174
Satd. Flow (prot)	0	1736	0	1770	1863	1583	1770	3539	1583	1770	3454	0
Flt Permitted		0.984		0.950			0.092			0.229		
Satd. Flow (perm)	0	1736	0	1770	1863	1583	171	3539	1583	427	3454	0
Satd. Flow (RTOR)		21				236			236		22	
Lane Group Flow (vph)	0	158	0	92	65	23	229	931	236	241	1289	0
Turn Type	Split	NA		Split	NA	Free	pm+pt	NA	Free	pm+pt	NA	
Protected Phases	3	3		4	4		1	6		5	2	
Permitted Phases						Free	6		Free	2		
Total Split (s)	20.0	20.0		16.0	16.0		25.0	63.0		26.0	64.0	
Total Lost Time (s)		7.0		7.0	7.0		8.0	8.0		8.0	8.0	
Act Effct Green (s)		12.3		8.7	8.7	121.4	71.9	57.7	121.4	68.7	56.1	
Actuated g/C Ratio		0.10		0.07	0.07	1.00	0.59	0.48	1.00	0.57	0.46	
v/c Ratio		0.81		0.73	0.49	0.01	0.79	0.55	0.15	0.63	0.80	
Control Delay		76.9		87.2	68.2	0.0	45.4	24.8	0.2	17.9	32.8	
Queue Delay		0.0		0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Total Delay		76.9		87.2	68.2	0.0	45.4	24.8	0.2	17.9	32.8	
LOS		Е		F	Е	Α	D	С	Α	В	С	
Approach Delay		76.9			69.2			24.0			30.5	
Approach LOS		Е			E			С			С	
Queue Length 50th (ft)		109		74	51	0	105	270	0	74	457	
Queue Length 95th (ft)		#205		#148	95	0	188	338	0	105	520	
Internal Link Dist (ft)		2234			595			1612			403	
Turn Bay Length (ft)				125		300	200		500	200		
Base Capacity (vph)		204		131	138	1583	329	1681	1583	459	1607	
Starvation Cap Reductn		0		0	0	0	0	0	0	0	0	
Spillback Cap Reductn		0		0	0	0	0	0	0	0	0	
Storage Cap Reductn		0		0	0	0	0	0	0	0	0	
Reduced v/c Ratio		0.77		0.70	0.47	0.01	0.70	0.55	0.15	0.53	0.80	
Intersection Summary												
Cycle Length: 125												
Actuated Cycle Length: 121.4												
Control Type: Actuated-Uncoo	rdinated											
Maximum v/c Ratio: 0.81												
Intersection Signal Delay: 32 1				In	tersection	110S-C						

Intersection Signal Delay: 32.1 Intersection Capacity Utilization 75.9% Intersection LOS: C ICU Level of Service D

Analysis Period (min) 15

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

Splits and Phases: 1: FM 2818 & F and B Road/HSC Parkway



Synchro 10 Report 06/06/2018

Intersection						
Intersection Delay, s/veh	12.1					
Intersection LOS	В					
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	<u></u>	T T	WDL	4	W	HUIL
Traffic Vol, veh/h	127	91	69	322	81	30
Future Vol, veh/h	127	91	69	322	81	30
Peak Hour Factor	0.86	0.86	0.86	0.86	0.86	0.86
Heavy Vehicles, %	2	2	2	2	2	2
Mymt Flow	148	106	80	374	94	35
Number of Lanes	140	100	0	1	1	0
		'	-	'		U
Approach	EB		WB		NB	
Opposing Approach	WB		EB			
Opposing Lanes	1		2		0	
Conflicting Approach Left			NB		EB	
Conflicting Lanes Left	0		1		2	
Conflicting Approach Right	NB				WB	
Conflicting Lanes Right	1		0		1	
HCM Control Delay	8.9		14.5		9.9	
HCM LOS	Α		В		Α	
Lane		NBLn1	EBLn1	FBI n2	WBLn1	
Vol Left, %		73%	0%	0%	18%	
Vol Thru, %		0%	100%	0%	82%	
Vol Right, %		27%	0%	100%	0%	
Sign Control		Stop	Stop	Stop	Stop	
Traffic Vol by Lane		111	127	91	391	
LT Vol		81	0	0	69	
Through Vol		0	127	0	322	
RT Vol		30	0	91	0	
Lane Flow Rate		129	148	106	455	
Geometry Grp		2	7	7	5	
Degree of Util (X)		0.197	0.216	0.134	0.594	
Departure Headway (Hd)		5.491	5.272	4.566	4.706	
Convergence, Y/N		Yes	Yes	Yes	Yes	
Cap		649	678	780	764	
Service Time		3.562	3.03	2.323	2.753	
HCM Lane V/C Ratio		0.199	0.218	0.136	0.596	
HCM Control Delay		9.9	9.5	0.130	14.5	
HCM Control Delay HCM Lane LOS		9.9 A	9.5 A	A A	14.5 B	
HOW LAIR LUS		А	А	A	В	

Synchro 10 Report 06/06/2018

0.5

0.7 0.8

Int Delay, s/veh													
Movement	Intersection												
Lane Configurations	Int Delay, s/veh	3.5											
Lane Configurations		FRI	FRT	FRP	WRI	WRT	WRP	MRI	NRT	NRP	SRI	SRT	SRP
Traffic Vol, veh/h				LDIX	WDL		WDIX	NDL		NDIX	JDL		JUK
Future Vol, veh/h Conflicting Peds, #/hr O O O O O O O O O O O O O O O O O O O				٥	٥		36	٥		Λ	116		2/
Conflicting Peds, #hr 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0				-	_							-	
Sign Control Free					_				-				
RT Channelized		_		-	-	-	-		-			_	
Storage Length 100		1100			1100				Jiop -		Jiop -		
Veh in Median Storage, # 0 - 0 0 - 0 0 - 0 0 - 0 0 - 0 0 - 0 0 - 0 0 - 0 0 - 0 0 - 0 0 - 0 <td></td> <td>100</td> <td></td> <td>-</td> <td></td> <td></td> <td>-</td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td>		100		-			-						
Grade, %			0			0			0			0	
Peak Hour Factor			-			-			-				
Heavy Vehicles, % 2 2 2 2 2 2 2 2 2		89				_							89
Major/Minor Major1 Major2 Minor1 Minor2 Minor2 Minor3 Major4 Major4 Major5 Minor4 Minor5 Minor5 Minor5 Minor6 Major6 Minor6 Minor6 Minor6 Minor6 Minor6 Minor7 Minor6 Minor7 Minor6 Minor7 Minor8 Minor		2	2	2	2	2	2	2	2	2	2	2	2
Major/Minor Major1 Major2 Minor1 Minor2 Conflicting Flow All 495 0 0 146 0 0 386 653 73 560 633 248 Stage 1 - - - - - 158 158 - 475 475 475 475 518 - 475 465 4694 7.54 6.54 6.54 6.54 6.54 6.54 5.54 -6.54 5.54 -6.54 5.54 -6.54 5.54 -6.54 5.54 -6.54 5.54 -6.54 5.54 -6.54 5.54 -6.54 5.54 -6.													
Conflicting Flow All 495 0 0 146 0 0 386 653 73 560 633 248													
Conflicting Flow All	Major/Minor A	Anior1			Anior?		,	Ainor1		N	linor?		
Stage 1			۸			0			452			622	2/10
Stage 2				U									
Critical Hdwy 4.14 - 4.14 - 4.14 - 7.54 6.54 6.94 7.54 6.54 6.94 Critical Hdwy Stg 1 - - - - 6.54 5.54 <td></td> <td></td> <td></td> <td>-</td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td>				-									
Critical Hdwy Stg 1 - - - - 6.54 5.54 - 3.32 3.52 4.02 3.32 3.52 4.02 3.32 7.62 - 5.44 - 1.35 7.66 - 5.29 58 - - 5.25 383 974 411 395 7.52 - 8.02 761 - 5.36 5.56 - - 5.25 383 974					111								
Critical Hdwy Stg 2 - - - 6.54 5.54 - 6.54 5.54 - 5.54 - 5.54 - 5.54 - 5.54 - 5.54 - 5.54 - 5.54 - 5.54 - 5.54 - 5.54 - 5.54 - 5.54 - 5.54 - 5.52 3.82 7.64 1.02 3.95 7.52 7.52 7.52 7.54 5.44 - 9.13 7.66 - - 5.25 383 9.74 4.09 393 7.52 -					4.14								
Follow-up Hdwy 2.22 - 2.22 - 3.52 4.02 3.32 3.52 4.02 3.32 Pol Cap-1 Maneuver 1065 - 1434 - 547 385 974 411 395 752 Stage 1 828 766 - 539 556 - Stage 2 754 544 - 913 766 - Platoon blocked, %													
Pot Cap-1 Maneuver 1065 - 1434 - 547 385 974 411 395 752 Stage 1 - - - - 828 766 - 539 556 - Stage 2 - - - - 754 544 - 913 766 - Platoon blocked, % - <td></td> <td>2 22</td> <td></td> <td></td> <td>2 22</td> <td></td> <td></td> <td></td> <td></td> <td>3 32</td> <td></td> <td></td> <td></td>		2 22			2 22					3 32			
Stage 1 - - - 828 766 - 539 556 - Stage 2 - - - 754 544 - 913 766 - Platoon blocked, % -													
Stage 2 -					1101								
Platoon blocked, % -													
Mov Cap-1 Maneuver 1065 - 1434 - 525 383 974 409 393 752 Mov Cap-2 Maneuver - - - - 525 383 - 409 393 - Stage 1 - - - - - 823 761 - 536 556 - Stage 2 - - - - 727 544 - 908 761 - Approach EB WB NB SB - <td></td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td> <td>.01</td> <td></td> <td></td> <td>. 10</td> <td>.00</td> <td></td>								.01			. 10	.00	
Mov Cap-2 Maneuver - - - 525 383 409 393 - Stage 1 - - - 823 761 - 536 556 - Stage 2 - - - - 727 544 908 761 - Approach EB WB NB SB B B B HCM Control Delay, s 0.3 0 0 17.5 HCM Control Delay, s 0.3 C C MINOR Lane/Major Mvmt NBLn1 EBL EBT EBR WBL WBT WBR SBLn1 Capacity (velvh) - 1065 - 1434 - 444 HCM Lane V/C Ratio - 0.005 - - - 0.354 -		1065			1434			525	383	974	409	393	752
Stage 1 - - - - 823 761 - 536 556 - Stage 2 - - - - 727 544 - 908 761 - Approach EB WB NB SB -				-		-	-						
Stage 2 - - - - 727 544 - 908 761 - Approach EB WB NB SB HCM Control Delay, s 0.3 0 0 17.5 HCM LOS A C Minor Lane/Major Mvmt NBLn1 EBL EBT EBR WBL WBT SBLn1 Capacity (velvh) - 1065 - 1434 - 444 HCM Lane V/C Ratio - 0.005 - - - - 0.354												556	
Approach EB WB NB SB HCM Control Delay, s 0.3 0 0 17.5 HCM LOS A C Minor Lane/Major Mvmt NBLn1 EBL EBT EBR WBL WBT WBR SBLn1 Capacity (velv/h) - 1065 - 1434 - 444 HCM Lane V/C Ratio - 0.005 0.354				-	-	-	-			-			
HCM Control Delay, s 0.3 0 0 17.5 HCM LOS A C Minor Lane/Major Mymt NBLn1 EBL EBT EBR WBL WBT WBR SBLn1 Capacity (vel/th) - 1065 - 1434 - 444 HCM Lane V/C Ratio - 0.005 0.354	J												
HCM Control Delay, s 0.3 0 0 17.5 HCM LOS A C Minor Lane/Major Mymt NBLn1 EBL EBT EBR WBL WBT WBR SBLn1 Capacity (vel/th) - 1065 - 1434 - 444 HCM Lane V/C Ratio - 0.005 0.354	Annroach	EP			WP			MP			SP		
HCM LOS													
Minor Lane/Major Mvmt NBLn1 EBL EBR WBL WBT WBR SBLn1 Capacity (vel/h) - 1065 - - 1434 - - 444 HCM Lane V/C Ratio - 0.005 - - - - 0.354		0.3			U								
Capacity (velvh) - 1065 - - 1434 - - 444 HCM Lane V/C Ratio - 0.005 - - - - 0.354	HCIVI LUS							А			C		
Capacity (velvh) - 1065 - - 1434 - - 444 HCM Lane V/C Ratio - 0.005 - - - - 0.354													
HCM Lane V/C Ratio - 0.005 0.354		i I				EBR							
			-		-	-	1434	-					
						-		-	-				
	HCM Control Delay (s)		0	8.4	-	-	0	-	-	17.5			
HCM Lane LOS A A A C			Α		-	-		-	-				
HCM 95th %tile Q(veh) - 0 0 1.6	HCM 95th %tile Q(veh)		-	0	-	-	0	-	-	1.6			

Intersection												
Intersection Delay, s/veh	9.1											
Intersection LOS	Α											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBF
Lane Configurations	7	ħβ		ሻ	↑ 1>			4			4	
Traffic Vol, veh/h	0	51	10	98	219	95	2	0	19	63	6	19
Future Vol, veh/h	0	51	10	98	219	95	2	0	19	63	6	19
Peak Hour Factor	0.87	0.87	0.87	0.87	0.87	0.87	0.87	0.87	0.87	0.87	0.87	0.87
Heavy Vehicles, %	2	2	2	2	2	2	2	2	2	2	2	2
Mvmt Flow	0	59	11	113	252	109	2	0	22	72	7	2:
Number of Lanes	1	2	0	1	2	0	0	1	0	0	1	(
Approach	EB			WB			NB			SB		
Opposing Approach	WB			EB			SB			NB		
Opposing Lanes	3			3			1			1		
Conflicting Approach Left	SB			NB			EB			WB		
Conflicting Lanes Left	1			1			3			3		
Conflicting Approach Right	NB			SB			WB			EB		
Conflicting Lanes Right	1			1			3			3		
HCM Control Delay	8.3			9.1			8.2			9.9		
HCM LOS	А			Α			Α			Α		
Lane		NBLn1	EBLn1	EBLn2	EBLn3	WBLn1	WBLn2	WBLn3	SBLn1			
Vol Left, %		10%	0%	0%	0%	100%	0%	0%	72%			
Vol Thru, %		0%	100%	100%	63%	0%	100%	43%	7%			
Vol Right, %		90%	0%	0%	37%	0%	0%	57%	22%			
Sign Control		Stop	Stop	Stop	Stop	Stop	Stop	Stop	Stop			
Traffic Vol by Lane		21	0	34	27	98	146	168	88			
LT Vol		2	0	0	0	98	0	0	63			
Through Vol		0	0	34	17	0	146	73	6			
RT Vol		19	0	0	10	0	0	95	19			
Lane Flow Rate		24	0	39	31	113	168	193	101			
Geometry Grp		7	7	7	7	7	7	7	7			
Degree of Util (X)		0.035	0	0.058	0.044	0.171	0.232	0.245	0.167			
Departure Headway (Hd)		5.268	5.351	5.351	5.09	5.473	4.971	4.573	5.942			
Convergence, Y/N		Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes			
Cap		677	0	667	701	656	722	786	603			
Service Time		3.02	3.098	3.098	2.836	3.203	2.701	2.303	3.686			
HCM Cantral Dalan		0.035	0	0.058	0.044	0.172 9.3	0.233	0.246	0.167			
HCM Control Delay HCM Lane LOS		8.2 A	8.1 N	8.4 A	8.1 A	9.3 A	9.2 A	8.8 A	9.9 A			
HCM Lane LOS HCM 95th-tile Q		0.1	0	0.2	0.1	0.6	0.9	A 1	0.6			
LICINI ADILI-IIIE C		U. I	U	0.2	U. I	0.6	0.9	- 1	0.0			

Intersection	
Intersection Delay, s/veh Intersection LOS	8.8
Intersection LOS	Α

Movement	FRL	FRI	FBK	WBL	WBI	WBR	NBL	MRI	NRK	SBL	SRI	SRK
Lane Configurations		4						^	7	7	^	
Traffic Vol, veh/h	43	5	7	0	0	0	0	5	16	122	30	0
Future Vol, veh/h	43	5	7	0	0	0	0	5	16	122	30	0
Peak Hour Factor	0.75	0.75	0.75	0.75	0.75	0.75	0.75	0.75	0.75	0.75	0.75	0.75
Heavy Vehicles, %	2	2	2	2	2	2	2	2	2	2	2	2
Mvmt Flow	57	7	9	0	0	0	0	7	21	163	40	0
Number of Lanes	0	1	0	0	0	0	0	1	1	1	2	0
Approach	EB							NB		SB		
Opposing Approach								SB		NB		
Opposing Lanes	0							3		2		
Conflicting Approach Left	SB							EB				
Conflicting Lanes Left	3							1		0		
Conflicting Approach Right	NB									EB		
Conflicting Lanes Right	2							0		1		
HCM Control Delay	8.8							7.4		9		
HCM LOS	Α							Α		Α		

Lane	NBLn1	NBLn2	EBLn1	SBLn1	SBLn2	SBLn3
Vol Left, %	0%	0%	78%	100%	0%	0%
Vol Thru. %	100%	0%	9%	0%	100%	100%
Vol Right, %	0%	100%	13%	0%	0%	0%
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop
Traffic Vol by Lane	5	16	55	122	15	15
LT Vol	0	0	43	122	0	0
Through Vol	5	0	5	0	15	15
RT Vol	0	16	7	0	0	0
Lane Flow Rate	7	21	73	163	20	20
Geometry Grp	8	8	7	7	7	7
Degree of Util (X)	0.01	0.027	0.11	0.234	0.026	0.016
Departure Headway (Hd)	5.176	4.474	5.392	5.179	4.678	2.942
Convergence, Y/N	Yes	Yes	Yes	Yes	Yes	Yes
Cap	694	803	668	685	754	1185
Service Time	2.886	2.183	3.097	2.978	2.477	0.738
HCM Lane V/C Ratio	0.01	0.026	0.109	0.238	0.027	0.017
HCM Control Delay	7.9	7.3	8.8	9.6	7.6	5.8
HCM Lane LOS	Α	Α	Α	Α	Α	Α
HCM 95th-tile Q	0	0.1	0.4	0.9	0.1	0

Intersection												
Intersection Delay, s/veh	8.5											
Intersection LOS	Α											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4		7	•			4	
Traffic Vol, veh/h	3	0	6	23	9	85	22	27	0	0	113	65
Future Vol, veh/h	3	0	6	23	9	85	22	27	0	0	113	65
Peak Hour Factor	0.79	0.79	0.79	0.79	0.79	0.79	0.79	0.79	0.79	0.79	0.79	0.79
Heavy Vehicles, %	2	2	2	2	2	2	2	2	2	2	2	2
Mvmt Flow	4	0	8	29	11	108	28	34	0	0	143	82
Number of Lanes	0	1	0	0	1	0	1	1	0	0	1	0
Approach	EB			WB			NB				SB	
Opposing Approach	WB			EB			SB				NB	
Opposing Lanes	1			1			1				2	
Conflicting Approach Left	SB			NB			EB				WB	
Conflicting Lanes Left	1			2			1				1	
Conflicting Approach Right	NB			SB			WB				EB	
Conflicting Lanes Right	2			1			1				1	
HCM Control Delay	7.5			8.1			8.2				8.8	
HCM LOS	Α			Α			Α				Α	

Lane	NBLn1	NBLn2	EBLn1	WBLn1	SBLn1
Vol Left, %	100%	0%	33%	20%	0%
Vol Thru, %	0%	100%	0%	8%	63%
Vol Right, %	0%	0%	67%	73%	37%
Sign Control	Stop	Stop	Stop	Stop	Stop
Traffic Vol by Lane	22	27	9	117	178
LT Vol	22	0	3	23	0
Through Vol	0	27	0	9	113
RT Vol	0	0	6	85	65
Lane Flow Rate	28	34	11	148	225
Geometry Grp	7	7	2	2	5
Degree of Util (X)	0.043	0.048	0.014	0.173	0.266
Departure Headway (Hd)	5.562	5.059	4.422	4.204	4.245
Convergence, Y/N	Yes	Yes	Yes	Yes	Yes
Cap	645	709	810	855	847
Service Time	3.284	2.781	2.446	2.221	2.262
HCM Lane V/C Ratio	0.043	0.048	0.014	0.173	0.266
HCM Control Delay	8.5	8	7.5	8.1	8.8
HCM Lane LOS	Α	Α	Α	Α	Α
HCM 95th-tile Q	0.1	0.2	0	0.6	1.1

Loveless Tract TIA Background (2021) 1: FM 2818 & F and B Road/F&B Road PM Peak Hour

	*	-	\rightarrow	•	←	*	4	†	1	-	↓	4
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4		Ĭ	*	7	7	^	7	7	↑ ↑	
Traffic Volume (vph)	140	112	140	193	79	120	193	1324	140	52	912	162
Future Volume (vph)	140	112	140	193	79	120	193	1324	140	52	912	162
Satd. Flow (prot)	0	1741	0	1770	1863	1583	1770	3539	1583	1770	3458	0
Flt Permitted		0.982		0.950			0.078			0.090		
Satd. Flow (perm)	0	1741	0	1770	1863	1583	145	3539	1583	168	3458	0
Satd. Flow (RTOR)		20				236			236		18	
Lane Group Flow (vph)	0	426	0	210	86	130	210	1439	152	57	1167	0
Turn Type	Split	NA		Split	NA	Free	pm+pt	NA	Free	pm+pt	NA	
Protected Phases	3	3		4	4		1	6		5	2	
Permitted Phases						Free	6		Free	2		
Total Split (s)	34.0	34.0		21.0	21.0		19.0	57.0		13.0	51.0	
Total Lost Time (s)		7.0		7.0	7.0		8.0	8.0		8.0	8.0	
Act Effct Green (s)		27.0		14.0	14.0	125.0	60.4	51.6	125.0	48.0	43.0	
Actuated g/C Ratio		0.22		0.11	0.11	1.00	0.48	0.41	1.00	0.38	0.34	
v/c Ratio		1.09		1.06	0.41	0.08	0.99	0.98	0.10	0.45	0.97	
Control Delay		115.5		133.5	58.3	0.1	91.8	57.4	0.1	29.1	60.0	
Queue Delay		0.0		0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Total Delay		115.5		133.5	58.3	0.1	91.8	57.4	0.1	29.1	60.0	
LOS		F		F	Е	Α	F	Е	Α	С	E	
Approach Delay		115.5			77.6			56.5			58.6	
Approach LOS		F			Ε			Е			E	
Queue Length 50th (ft)		~374		~186	66	0	123	~659	0	23	481	
Queue Length 95th (ft)		#581		#345	120	0	#286	#799	0	47	#634	
Internal Link Dist (ft)		2234			595			1612			403	
Turn Bay Length (ft)				125		300	200		500	200		
Base Capacity (vph)		391		198	208	1583	212	1461	1583	128	1201	
Starvation Cap Reductn		0		0	0	0	0	0	0	0	0	
Spillback Cap Reductn		0		0	0	0	0	0	0	0	0	
Storage Cap Reductn		0		0	0	0	0	0	0	0	0	
Reduced v/c Ratio		1.09		1.06	0.41	0.08	0.99	0.98	0.10	0.45	0.97	

Intersection Summary

Cycle Length: 125
Actuated Cycle Length: 125
Control Type: Actuated-Uncoordinated

Maximum v/c Ratio: 1.09

Intersection Signal Delay: 66.0
Intersection Capacity Utilization 89.1% Intersection LOS: E ICU Level of Service E

Analysis Period (min) 15

Volume exceeds capacity, queue is theoretically infinite.
 Queue shown is maximum after two cycles.

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.



Loveless Tract TIA Background (2021) 2: Turkey Creek Road & HSC Parkway/F and B Road PM Peak Hour

Intersection							
Intersection Delay, s/veh	17.8						
Intersection LOS	С						
Movement	EBT	EBR	WBL	WBT	NBL	NBR	
Lane Configurations	†	7		4	¥		
Traffic Vol, veh/h	333	161	191	278	78	45	
Future Vol, veh/h	333	161	191	278	78	45	
Peak Hour Factor	0.88	0.88	0.88	0.88	0.88	0.88	
Heavy Vehicles, %	2	2	2	2	2	2	
Mymt Flow	378	183	217	316	89	51	
Number of Lanes	1	103	0	1	1	0	
		'		'		U	
Approach	EB		WB		NB		
Opposing Approach	WB		EB				
Opposing Lanes	1		2		0		
Conflicting Approach Left			NB		EB		
Conflicting Lanes Left	0		1		2		
Conflicting Approach Right	NB				WB		
Conflicting Lanes Right	1		0		1		
HCM Control Delay	13.6		23.9		11.4		
HCM LOS	В		С		В		
Lano		NBLn1	EBLn1	EBLn2	WBLn1		
Lane		63%	0%	O%	41%		
Vol Left, %		0%		0%	41% 59%		
Vol Thru, %			100%				
Vol Right, %		37% Stop	0%	100%	0%		
Sign Control		Stop	Stop	Stop	Stop		
Traffic Vol by Lane		123	333	161	469		
LT Vol		78	0	0	191		
Through Vol		0	333	0	278		
RT Vol		45	0	161	0		
Lane Flow Rate		140	378	183	533		
Geometry Grp		2	7	7	5		
Degree of Util (X)		0.244	0.583	0.247	0.775		
Departure Headway (Hd)		6.28	5.546	4.852	5.237		
Convergence, Y/N		Yes	Yes	Yes	Yes		
Cap		572	651	743	693		
Service Time		4.322	3.273	2.565	3.239		
HCM Lane V/C Ratio		0.245	0.581	0.246	0.769		
HCM Control Delay		11.4	15.8	9.2	23.9		
HCM Lane LOS		В	С	Α	С		
			3.8		7.5		

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Intersection	0.1											
Int Delay, s/veh	2.4											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	7	∱ ⊅		7	† 1>			4			4	
Traffic Vol, veh/h	25	322	0	0	206	112	0	0	0	82	0	14
Future Vol, veh/h	25	322	0	0	206	112	0	0	0	82	0	14
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None		-	None	-		None
Storage Length	100	-	-	150	-	-	-	-	-	-	-	-
Veh in Median Storage,	# -	0	-	-	0	-		0	-	-	0	
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	83	83	83	83	83	83	83	83	83	83	83	83
Heavy Vehicles, %	2	2	2	2	2	2	2	2	2	2	2	2
Mvmt Flow	30	388	0	0	248	135	0	0	0	99	0	17
Major/Minor M	lajor1		N	/lajor2		N	/linor1		Λ	/linor2		
Conflicting Flow All	383	0	0	388	0	0	572	831	194	570	764	192
Stage 1	-		-	-	-	-	448	448		316	316	
Stage 2							124	383		254	448	
Critical Hdwy	4.14			4.14			7.54	6.54	6.94	7.54	6.54	6.94
Critical Hdwy Stg 1							6.54	5.54	-	6.54	5.54	
Critical Hdwy Stg 2			-	-		-	6.54	5.54	-	6.54	5.54	-
Follow-up Hdwy	2.22			2.22			3.52	4.02	3.32	3.52	4.02	3.32
	1172			1167			403	304	815	404	332	817
Stage 1			-	-		-	560	571	-	670	654	-
Stage 2	-		-				867	610	-	728	571	
Platoon blocked, %			-									
	1172			1167			387	296	815	396	323	817
Mov Cap-2 Maneuver	-	-	-	-	-	-	387	296		396	323	
Stage 1	-	-			-	-	545	556		653	654	
Stage 2		-	-	-	-	-	849	610		709	556	-
, , ,												
Approach	EB			WB			NB			SB		
HCM Control Delay, s	0.6			0			0			16.5		
HCM LOS	0.0			U			A			C		
TICIVI EOS							Α.			C		
		IDI 4	ED!	COT	EDD	MDI	MOT	WDD	201 4			
Minor Lane/Major Mvmt	Г	VBLn1	EBL	EBT	EBR	WBL	WBT	WBR :				
Capacity (veh/h)		-	1172	-	-	1167	-	-	428 0.27			
LICHTI VIC D. "							-					
		-	0.026		-							
		0	8.2	-		0		-	16.5			
HCM Lane V/C Ratio HCM Control Delay (s) HCM Lane LOS HCM 95th %tile Q(veh)					-							

latana atian												
Intersection	0.5											
Intersection Delay, s/veh	9.5											
Intersection LOS	Α											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBF
Lane Configurations	7	↑ 1>		*	† 1>			4			44	
Traffic Vol, veh/h	14	124	7	21	79	137	19	2	90	122	5	10
Future Vol, veh/h	14	124	7	21	79	137	19	2	90	122	5	10
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles, %	2	2	2	2	2	2	2	2	2	2	2	
Mvmt Flow	15	135	8	23	86	149	21	2	98	133	5	1
Number of Lanes	1	2	0	1	2	0	0	1	0	0	1	(
Approach	EB			WB			NB			SB		
Opposing Approach	WB			EB			SB			NB		
Opposing Lanes	3			3			1			1		
Conflicting Approach Left	SB			NB			EB			WB		
Conflicting Lanes Left	1			1			3			3		
Conflicting Approach Right	NB			SB			WB			EB		
Conflicting Lanes Right	1			1			3			3		
HCM Control Delay	9.1			9.1			9.1			10.8		
HCM LOS	Α			Α			Α			В		
Lane		NBLn1	EBLn1	EBLn2	EBLn3	WBLn1	WBLn2	WBLn3	SBLn1			
Vol Left, %		17%	100%	0%	0%	100%	0%	0%	89%			
Vol Thru, %		2%	0%	100%	86%	0%	100%	16%	4%			
Vol Right, %		81%	0%	0%	14%	0%	0%	84%	7%			
Sign Control		Stop	Stop	Stop	Stop	Stop	Stop	Stop	Stop			
Traffic Vol by Lane		111	14	83	48	21	53	163	137			
LT Vol		19	14	0	0	21	0	0	122			
Through Vol		2	0	83	41	0	53	26	5			
RT Vol		90	0	0	7	0	0	137	10			
Lane Flow Rate		121	15	90	53	23	57	178	149			
Geometry Grp		7	7	7	7	7	7	7	7			
Degree of Util (X)		0.174	0.026	0.139	0.08	0.038	0.087	0.241	0.249			
Departure Headway (Hd)		5.206	6.085	5.579	5.476	5.993	5.487	4.893	6.016			
Convergence, Y/N		Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes			
Cap		682	584	637	648	594	648	728	593			
Service Time		2.994	3.868	3.361	3.259	3.767	3.261	2.667	3.799			
HCM Lane V/C Ratio		0.177	0.026	0.141	0.082	0.039	0.088	0.245	0.251			
HCM Control Delay		9.1	9	9.3	8.7	9	8.8	9.2	10.8			
HCM Lane LOS		A	A	A	A	A	A	A	В			
HCM 95th-tile Q		0.6	0.1	0.5	0.3	0.1	0.3	0.9	1			

Intersection	0.0											
Intersection Delay, s/veh	8.9											
ntersection LOS	Α											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4						^	7	ħ	^	
Traffic Vol, veh/h	39	7	5	0	0	0	0	8	46	160	68	C
Future Vol, veh/h	39	7	5	0	0	0	0	8	46	160	68	C
Peak Hour Factor	0.82	0.82	0.82	0.82	0.82	0.82	0.82	0.82	0.82	0.82	0.82	0.82
Heavy Vehicles, %	2	2	2	2	2	2	2	2	2	2	2	2
Mvmt Flow	48	9	6	0	0	0	0	10	56	195	83	C
Number of Lanes	0	1	0	0	0	0	0	1	1	1	2	C
Approach	EB							NB		SB		
Opposing Approach								SB		NB		
Opposing Lanes	0							3		2		
Conflicting Approach Left	SB							EB				
Conflicting Lanes Left	3							1		0		
Conflicting Approach Right	NB									EB		
Conflicting Lanes Right	2							0		1		
HCM Control Delay	9							7.7		9.1		
HCM LOS	Α							Α		Α		
		NDI 4	NDI 0	EDI 4	CDL 4	CDL A	CDL A					
Lane		NBLn1	NBLn2	EBLn1	SBLn1	SBLn2	SBLn3					
/ol Left, %		0%	0%	76%	100%	0%	0%					
/ol Thru, %		100%	0%	14%	0%	100%	100%					
Vol Right, %		0%	100%	10%	0%	0%	0%					
Sign Control		Stop	Stop	Stop	Stop	Stop	Stop					
Traffic Vol by Lane		8	46	51	160	34	34					
LT Vol		0	0	39	160	0	0					
Through Vol		8	0	7	0	34	34					
RT Vol		0	46	5	0	0	0					
Lane Flow Rate		10	56	62	195	41	41					
Geometry Grp		8	8	7	7	7	7					
Degree of Util (X)		0.014	0.071	0.097	0.286	0.055	0.034					
Departure Headway (Hd)		5.274	4.572	5.635	5.28	4.779	2.941					
					Yes	Yes	Yes					
Convergence, Y/N		Yes	Yes	Yes								
Сар		680	785	638	685	754	1185					
Convergence, Y/N Cap Service Time												

0.015 0.071 0.097 0.285 0.054 0.035

10.1

1.2 0.2

7.8 5.8

8.1 7.6

0.2 0.3

Intersection												
Intersection Delay, s/veh	9.8											
Intersection LOS	Α											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBF
Lane Configurations		4			4		7	^			4	
Traffic Vol, veh/h	2	0	3	87	3	75	8	42	0	0	137	190
Future Vol, veh/h	2	0	3	87	3	75	8	42	0	0	137	190
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles, %	2	2	2	2	2	2	2	2	2	2	2	2
Mvmt Flow	2	0	3	97	3	83	9	47	0	0	152	211
Number of Lanes	0	1	0	0	1	0	1	1	0	0	1	(
Approach	EB			WB			NB				SB	
Opposing Approach	WB			EB			SB				NB	
Opposing Lanes	1			1			1				2	
Conflicting Approach Left	SB			NB			EB				WB	
Conflicting Lanes Left	1			2			1				1	
Conflicting Approach Right	NB			SB			WB				EB	
Conflicting Lanes Right	2			1			1				1	
HCM Control Delay	7.9			9.2			8.4				10.4	
HCM LOS	Α			Α			Α				В	
Lane		NBLn1	NBLn2	EBLn1	WBLn1	SBLn1						
Vol Left, %		100%	0%	40%	53%	0%						
Vol Thru, %		0%	100%	0%	2%	42%						
Vol Right, %		0%	0%	60%	45%	58%						
Sign Control		Stop	Stop	Stop	Stop	Stop						
Traffic Vol by Lane		8	42	5	165	327						
LT Vol		8	0	2	87	0						
Through Vol		0	42	0	3	137						
RT Vol		0	0	3	75	190						
Lane Flow Rate		9	47	6	183	363						
Geometry Grp		7	7	2	2	5						
Degree of Util (X)		0.014	0.069	0.007	0.239	0.426						
Departure Headway (Hd)		5.792	5.288	4.825	4.699	4.22						
Convergence, Y/N		Yes	Yes	Yes	Yes	Yes						
Cap		618	677	739	763	854						
Service Time		3.529	3.025	2.875	2.734	2.245						
HOME VIO D-E-		0.045	0.070	0.000	0.04	0.405						

0.015 0.069 0.008 0.24 0.425

7.9

0 0.9 2.2

9.2 10.4

8.6 8.4

0

0.2

HCM Lane V/C Ratio

HCM Control Delay HCM Lane LOS

HCM 95th-tile Q

HCM Lane V/C Ratio

HCM Control Delay HCM Lane LOS

HCM 95th-tile Q

	۶	→	*	•	+	4	1	†	~	1	+	4
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4		ሻ	↑	7	7	^	7	ሻ	↑ ↑	
Traffic Volume (vph)	74	44	53	78	57	19	196	801	202	207	935	182
Future Volume (vph)	74	44	53	78	57	19	196	801	202	207	935	182
Satd. Flow (prot)	0	1747	0	1770	1863	1583	1770	3539	1583	1770	3454	0
Flt Permitted		0.979		0.950			0.078			0.217		
Satd. Flow (perm)	0	1747	0	1770	1863	1583	145	3539	1583	404	3454	0
Satd. Flow (RTOR)		15				236			236		23	
Lane Group Flow (vph)	0	199	0	91	66	22	228	931	235	241	1299	0
Turn Type	Split	NA		Split	NA	Free	pm+pt	NA	Free	pm+pt	NA	
Protected Phases	3	3		4	4		1	6		5	2	
Permitted Phases						Free	6		Free	2		
Total Split (s)	23.0	23.0		16.0	16.0		24.0	60.0		26.0	62.0	
Total Lost Time (s)		7.0		7.0	7.0		8.0	8.0		8.0	8.0	
Act Effct Green (s)		15.3		8.7	8.7	122.4	69.5	55.2	122.4	67.2	54.1	
Actuated g/C Ratio		0.12		0.07	0.07	1.00	0.57	0.45	1.00	0.55	0.44	
v/c Ratio		0.86		0.72	0.50	0.01	0.84	0.58	0.15	0.65	0.84	
Control Delay		81.6		87.2	69.1	0.0	56.3	27.5	0.2	20.2	36.7	
Queue Delay		0.0		0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Total Delay		81.6		87.2	69.1	0.0	56.3	27.5	0.2	20.2	36.7	
LOS		F		F	Е	Α	Е	С	Α	С	D	
Approach Delay		81.6			69.8			27.6			34.1	
Approach LOS		F			E			С			С	
Queue Length 50th (ft)		148		73	52	0	121	285	0	80	485	
Queue Length 95th (ft)		#260		#147	97	0	#228	356	0	113	543	
Internal Link Dist (ft)		2234			595			1612			403	
Turn Bay Length (ft)				125		300	200		500	200		
Base Capacity (vph)		241		130	137	1583	296	1597	1583	438	1538	
Starvation Cap Reductn		0		0	0	0	0	0	0	0	0	
Spillback Cap Reductn		0		0	0	0	0	0	0	0	0	
Storage Cap Reductn		0		0	0	0	0	0	0	0	0	
Reduced v/c Ratio		0.83		0.70	0.48	0.01	0.77	0.58	0.15	0.55	0.84	
Intersection Summary												
Cycle Length: 125												
Actuated Cycle Length: 122.4												
Control Type: Actuated-Uncod	ordinated											
Maximum v/c Ratio: 0.86												
Intersection Signal Delay: 36.	1			In	tersection	n LOS: D						
Intersection Capacity Utilization	on 78.0%			IC	U Level	of Service	e D					
Analysis Period (min) 15												
# 95th percentile volume ex	ceeds ca	pacity, qu	eue may	be longer	r.							
Queue shown is maximum												

Splits and Phases	: 1: FM 2818 & F and B Road/F	ISC Parkway			
▼ Ø1	↓ ø ₂		♣ _{Ø3}	₹ _{Ø4}	
24 s	62 s		23 s	16 s	
Ø5	↑ ø ₆				
26 s	60 s				

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Intersection	
Intersection Delay, s/veh	13
Intersection LOS	В

FBI	EBK	WBL	WBI	NBL	MRK	
1	7		ની	W		
133	97	69	332	96	59	
133	97	69	332	96	59	
0.86	0.86	0.86	0.86	0.86	0.86	
2	2	2	2	2	2	
155	113	80	386	112	69	
1	1	0	1	1	0	
EB		WB		NB		
WB		EB				
1		2		0		
		NB		EB		
0		1		2		
NB				WB		
1		0		1		
9.3		16		10.7		
Α		С		В		
	133 133 0.86 2 155 1 EB WB 1 0 NB	133 97 133 97 0.86 0.86 2 2 155 113 1 1 EB WB 1 0 NB 1 9.3	133 97 69 133 97 69 0.86 0.86 0.86 2 2 2 155 113 80 1 1 0 EB WB WB EB 1 2 NB 0 1 NB 0 1 NB 1 0 9.3 16	133 97 69 332 133 97 69 332 0.86 0.86 0.86 0.86 2 2 2 2 2 155 113 80 386 1 1 0 1 EB WB WB EB 1 2 NB 0 1 NB 0 1 NB 1 0 9.3 16	133 97 69 332 96 133 97 69 332 96 0.86 0.86 0.86 0.86 2 2 2 2 2 155 113 80 386 112 1 1 0 1 1 EB	133 97 69 332 96 59 133 97 69 332 96 59 0.86 0.86 0.86 0.86 0.86 2 2 2 2 2 2 2 155 113 80 386 112 69 1 1 0 1 1 0 EB WB NB WB EB 1 2 0 NB EB 0 1 2 WB 1 1 2 NB WB 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1

Lane	NBLn1	EBLn1	EBLn2	WBLn1
Vol Left, %	62%	0%	0%	17%
Vol Thru, %	0%	100%	0%	83%
Vol Right, %	38%	0%	100%	0%
Sign Control	Stop	Stop	Stop	Stop
Traffic Vol by Lane	155	133	97	401
LT Vol	96	0	0	69
Through Vol	0	133	0	332
RT Vol	59	0	97	0
Lane Flow Rate	180	155	113	466
Geometry Grp	2	7	7	5
Degree of Util (X)	0.275	0.234	0.149	0.631
Departure Headway (Hd)	5.483	5.458	4.751	4.871
Convergence, Y/N	Yes	Yes	Yes	Yes
Cap	649	651	746	737
Service Time	3.578	3.246	2.538	2.944
HCM Lane V/C Ratio	0.277	0.238	0.151	0.632
HCM Control Delay	10.7	9.9	8.4	16
HCM Lane LOS	В	Α	Α	С
HCM 95th-tile Q	1.1	0.9	0.5	4.5

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Intersection												
Int Delay, s/veh	4.5											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	- 1	ት ቤ			4î.			4			4	
Traffic Vol, veh/h	4	129	8	26	405	35	29	0	11	115	0	24
Future Vol, veh/h	4	129	8	26	405	35	29	0	11	115	0	24
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-		None	-	-	None	-	-	None	-	-	None
Storage Length	100	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage,	# -	0	-	-	0			0	-	-	0	
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	89	89	89	89	89	89	89	89	89	89	89	89
Heavy Vehicles, %	2	2	2	2	2	2	2	2	2	2	2	2
Mvmt Flow	4	145	9	29	455	39	33	0	12	129	0	27
	Najor1		1	Major2		- 1	Vinor1		N	/linor2		
Conflicting Flow All	494	0	0	154	0	0	444	710	77	614	695	247
Stage 1	-	-	-	-	-	-	158	158	-	533	533	-
Stage 2	-	-	-	-	-	-	286	552	-	81	162	-
Critical Hdwy	4.14		-	4.14			7.54	6.54	6.94	7.54	6.54	6.94
Critical Hdwy Stg 1	-	-	-	-	-	-	6.54	5.54	-	6.54	5.54	-
Critical Hdwy Stg 2	-		-	-	-		6.54	5.54	-	6.54	5.54	-
Follow-up Hdwy	2.22	-	-	2.22	-	-	3.52	4.02	3.32	3.52	4.02	3.32
Pot Cap-1 Maneuver	1066	-	-	1424	-	-	497	357	968	376	364	753
Stage 1	-	-	-	-	-	-	828	766	-	498	523	-
Stage 2	-	-	-	-	-	-	697	513	-	918	763	-
Platoon blocked, %		-	-		-	-						
Mov Cap-1 Maneuver	1066	-	-	1424	-	-	468	346	968	362	352	753
Mov Cap-2 Maneuver	-	-	-	-	-	-	468	346	-	362	352	-
Stage 1	-	-	-	-	-	-	825	763		496	508	-
Stage 2	-	-	-	-	-	-	653	499	-	903	760	-
Approach	EB			WB			NB			SB		
HCM Control Delay, s	0.2			0.5			12.2			19.8		
HCM LOS							В			С		
Minor Lane/Major Mvmt	t 1	VBLn1	EBL	EBT	EBR	WBL	WBT	WBR	SBLn1			
Capacity (veh/h)		545	1066	-	-	1424	-	-	398			
HCM Lane V/C Ratio			0.004			0.021			0.392			
HCM Control Delay (s)		12.2	8.4	-	-	7.6	0.1		19.8			
HCM Lane LOS		В	A			A	A		C			
HCM 95th %tile Q(veh)		0.3	0			0.1	-		1.8			
70111 701110 2(1011)		0.0	- 3			0.1			5			

Intersection												
Intersection Delay, s/veh	9.2											
Intersection LOS	Α											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBF
Lane Configurations	7	† }		ሻ	ħβ			4			4	
Traffic Vol, veh/h	0	59	10	98	248	94	2	0	18	62	5	18
Future Vol, veh/h	0	59	10	98	248	94	2	0	18	62	5	18
Peak Hour Factor	0.87	0.87	0.87	0.87	0.87	0.87	0.87	0.87	0.87	0.87	0.87	0.87
Heavy Vehicles, %	2	2	2	2	2	2	2	2	2	2	2	2
Mvmt Flow	0	68	11	113	285	108	2	0	21	71	6	21
Number of Lanes	1	2	0	1	2	0	0	1	0	0	1	(
Approach	EB			WB			NB			SB		
Opposing Approach	WB			EB			SB			NB		
Opposing Lanes	3			3			1			1		
Conflicting Approach Left	SB			NB			EB			WB		
Conflicting Lanes Left	1			1			3			3		
Conflicting Approach Right	NB			SB			WB			EB		
Conflicting Lanes Right	1			1			3			3		
HCM Control Delay	8.4			9.2			8.3			10		
HCM LOS	Α			Α			Α			Α		
Lane		NBLn1	EBLn1	EBLn2	EBLn3	WBLn1	WBLn2	WBLn3	SBLn1			
Vol Left, %		10%	0%	0%	0%	100%	0%	0%	73%			
Vol Thru, %		0%	100%	100%	66%	0%	100%	47%	6%			
Vol Right, %		90%	0%	0%	34%	0%	0%	53%	21%			
Sign Control		Stop	Stop	Stop	Stop	Stop	Stop	Stop	Stop			
Traffic Vol by Lane		20	0	39	30	98	165	177	85			
LT Vol		2	0	0	0	98	0	0	62			
Through Vol		0	0	39	20	0	165	83	5			
RT Vol		18	0	0	10	0	0	94	18			
Lane Flow Rate		23	0	45	34	113	190	203	98			
Geometry Grp		7	7	7	7	7	7	7	7			
Degree of Util (X)		0.034	0	0.067	0.049	0.171	0.262	0.259	0.164			
Departure Headway (Hd)		5.35	5.37	5.37	5.133	5.471	4.969	4.595	6.028			
Convergence, Y/N		Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes			
Cap		666	0	665	695	656	722	782	594			
Service Time		3.108	3.121	3.121	2.884	3.205	2.703	2.328	3.777			
HCM Lane V/C Ratio		0.035	0	0.068	0.049	0.172	0.263	0.26	0.165			
HCM Control Delay		8.3	8.1	8.5	8.2	9.3	9.5	8.9	10			
HCM Lane LOS		Α	N	Α	Α	Α	Α	Α	Α			
HCM 95th-tile Q		0.1	0	0.2	0.2	0.6	1	1	0.6			

Intersection	_		_	_								
	9.7											
Intersection Delay, s/veh	9.7 A											
Intersection LOS	А											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4						†	7	*	^	
Traffic Vol, veh/h	42	4	6	0	0	0	0	4	16	174	30	0
Future Vol. veh/h	42	4	6	0	0	0	0	4	16	174	30	0
Peak Hour Factor	0.75	0.75	0.75	0.75	0.75	0.75	0.75	0.75	0.75	0.75	0.75	0.75
Heavy Vehicles, %	2	2	2	2	2	2	2	2	2	2	2	2
Mymt Flow	56	5	8	0	0	0	0	5	21	232	40	0
Number of Lanes	0	1	0	0	0	0	0	1	1	1	2	0
Approach	EB							NB		SB		
Opposing Approach								SB		NB		
Opposing Lanes	0							3		2		
Conflicting Approach Left	SB							EB				
Conflicting Lanes Left	3							1		0		
Conflicting Approach Right	NB									EB		
Conflicting Lanes Right	2							0		1		
HCM Control Delay	9							7.6		10.1		
HCM LOS	Α							Α		В		
Lane		NBLn1	NBLn2	EBLn1	SBLn1	SBLn2	SBLn3					
Vol Left, %		0%	0%	81%	100%	0%	0%					
Vol Thru, %		100%	0%	8%	0%	100%	100%					
Vol Right, %		0%	100%	12%	0%	0%	0%					
Sign Control		Stop	Stop	Stop	Stop	Stop	Stop					
Traffic Vol by Lane		4	16	52	174	15	15					
LT Vol		0	0	42	174	0	0					
Through Vol		4	0	4	0	15	15					
RT Vol		0	16	6	0	0	0					
Lane Flow Rate		5	21	69	232	20	20					
Geometry Grp		8	8	7	7	7	7					
Degree of Util (X)		0.008	0.027	0.108	0.34	0.026	0.016					
Departure Headway (Hd)		5.314	4.612	5.588	5.273	4.672	2.935					
		Yes	Yes	Yes	Yes	Yes	Yes					
Convergence, Y/N					107	755	1187					
Convergence, Y/N Cap		675	777	643	687							
Convergence, Y/N Cap Service Time		3.036	2.333	3.304	2.973	2.471	0.733					
Convergence, Y/N Cap Service Time HCM Lane V/C Ratio		3.036 0.007	2.333 0.027	3.304 0.107	2.973 0.338	2.471 0.026	0.733 0.017					
Convergence, Y/N Cap Service Time HCM Lane V/C Ratio HCM Control Delay		3.036 0.007 8.1	2.333 0.027 7.5	3.304 0.107 9	2.973 0.338 10.7	2.471 0.026 7.6	0.733 0.017 5.8					
Convergence, Y/N Cap Service Time HCM Lane V/C Ratio		3.036 0.007	2.333 0.027	3.304 0.107	2.973 0.338	2.471 0.026	0.733 0.017					

Intersection												
Intersection Delay, s/veh	9.1											
Intersection LOS	Α											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBI
Lane Configurations		4			4		ሻ	*			44	
Traffic Vol, veh/h	3	0	5	23	9	100	21	26	0	0	166	6
Future Vol, veh/h	3	0	5	23	9	100	21	26	0	0	166	6
Peak Hour Factor	0.79	0.79	0.79	0.79	0.79	0.79	0.79	0.79	0.79	0.79	0.79	0.7
Heavy Vehicles, %	2	2	2	2	2	2	2	2	2	2	2	
Mvmt Flow	4	0	6	29	11	127	27	33	0	0	210	8
Number of Lanes	0	1	0	0	1	0	1	1	0	0	1	
Approach	EB			WB			NB				SB	
Opposing Approach	WB			EB			SB				NB	
Opposing Lanes	1			1			1				2	
Conflicting Approach Left	SB			NB			EB				WB	
Conflicting Lanes Left	1			2			1				1	
Conflicting Approach Right	NB			SB			WB				EB	
Conflicting Lanes Right	2			1			1				1	
HCM Control Delay	7.7			8.4			8.4				9.7	
HCM LOS	Α			Α			Α				Α	
Lane		NBLn1	NBLn2	EBLn1	WBLn1	SBLn1						
Vol Left, %		100%	0%	38%	17%	0%						
Vol Thru, %		0%	100%	0%	7%	72%						
Vol Right, %		0%	0%	62%	76%	28%						
Sign Control		Stop	Stop	Stop	Stop	Stop						
Traffic Vol by Lane		21	26	8	132	230						
LT Vol		21	0	3	23	0						
Through Vol		0	26	0	9	166						
RT Vol		0	0	5	100	64						
Lane Flow Rate		27	33	10	167	291						
Geometry Grp		7	7	2	2	5						
Degree of Util (X)		0.042	0.047	0.013	0.201	0.351						
Departure Headway (Hd)		5.672	5.168	4.641	4.332	4.344						
Convergence, Y/N		Yes	Yes	Yes	Yes	Yes						
Cap		632	693	771	829	830						
Service Time		3.402	2.899	2.673	2.354	2.367						
HCM Lane V/C Ratio		0.043	0.048	0.013	0.201	0.351						
HCM Control Delay		8.7	8.2	7.7	8.4	9.7						
HCM Lane LOS		Α	Α	Α	Α	Α						

0.1

0.7

HCM 95th-tile Q

Loveless	Tract TIA
Full Build	(2021)

Approach

HCM LOS

HCM Control Delay, s 10.3

В

7: Turkey Creek Road & Proposed Driveway AM Peak Hour

Intersection						
Int Delay, s/veh	2					
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	W			^	↑	
Traffic Vol, veh/h	29	47	0	130	184	0
Future Vol, veh/h	29	47	0	130	184	0
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage,	# 0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	32	51	0	141	200	0

Major/Minor	Minor2	Λ	Najor1	Ma	ijor2	
Conflicting Flow All	341	200	-	0	-	0
Stage 1	200	-	-	-	-	-
Stage 2	141	-	-	-	-	-
Critical Hdwy	6.42	6.22	-	-	-	-
Critical Hdwy Stg 1	5.42	-	-	-	-	-
Critical Hdwy Stg 2	5.42	-	-	-	-	-
Follow-up Hdwy	3.518	3.318	-	-	-	-
Pot Cap-1 Maneuver	655	841	0	-	-	0
Stage 1	834	-	0	-	-	0
Stage 2	886	-	0	-	-	0
Platoon blocked, %				-	-	
Mov Cap-1 Maneuver	655	841	-	-	-	-
Mov Cap-2 Maneuver	655	-	-	-	-	-
Stage 1	834	-	-	-	-	-
Stage 2	886	-	-	-	-	-

Minor Lane/Major Mymt	NBT EBLn1	SBT
Capacity (veh/h)	- 759	-
HCM Lane V/C Ratio	- 0.109	-
HCM Control Delay (s)	- 10.3	-
HCM Lane LOS	- B	
HCM 95th %tile Q(veh)	- 0.4	-

Synchro 10 Report 06/06/2018

Loveless Tract TIA Full Build (2021)

1: FM 2818 & F&B Road PM Peak Hour

	•	-	\rightarrow	•	+	*	1	†	1	-	↓	1
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4		ሻ	↑	7	7	^	7	ሻ	↑ ↑	
Traffic Volume (vph)	157	116	140	193	85	120	193	1324	140	52	912	192
Future Volume (vph)	157	116	140	193	85	120	193	1324	140	52	912	192
Satd. Flow (prot)	0	1743	0	1770	1863	1583	1770	3539	1583	1770	3447	0
Flt Permitted		0.981		0.950			0.078			0.088		
Satd. Flow (perm)	0	1743	0	1770	1863	1583	145	3539	1583	164	3447	0
Satd. Flow (RTOR)		19				236			236		22	
Lane Group Flow (vph)	0	449	0	210	92	130	210	1439	152	57	1200	0
Turn Type	Split	NA		Split	NA	Free	pm+pt	NA	Free	pm+pt	NA	
Protected Phases	3	3		4	4		1	6		5	2	
Permitted Phases						Free	6		Free	2		
Total Split (s)	35.0	35.0		21.0	21.0		17.0	57.0		12.0	52.0	
Total Lost Time (s)		7.0		7.0	7.0		8.0	8.0		8.0	8.0	
Act Effct Green (s)		28.0		14.0	14.0	125.0	58.6	51.4	125.0	48.0	44.0	
Actuated g/C Ratio		0.22		0.11	0.11	1.00	0.47	0.41	1.00	0.38	0.35	
v/c Ratio		1.11		1.06	0.44	0.08	1.14	0.99	0.10	0.50	0.98	
Control Delay		120.5		133.5	59.2	0.1	137.9	58.3	0.1	33.8	60.4	
Queue Delay		0.0		0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Total Delay		120.5		133.5	59.2	0.1	137.9	58.3	0.1	33.8	60.4	
LOS		F		F	Е	Α	F	Е	Α	С	Е	
Approach Delay		120.5			77.5			62.7			59.2	
Approach LOS		F			Е			Е			Е	
Queue Length 50th (ft)		~403		~186	71	0	~150	~659	0	24	494	
Queue Length 95th (ft)		#613		#345	127	0	#310	#799	0	48	#651	
Internal Link Dist (ft)		2234			595			1612			403	
Turn Bay Length (ft)				125		300	200		500	200		
Base Capacity (vph)		405		198	208	1583	185	1455	1583	114	1227	
Starvation Cap Reductn		0		0	0	0	0	0	0	0	0	
Spillback Cap Reductn		0		0	0	0	0	0	0	0	0	
Storage Cap Reductn		0		0	0	0	0	0	0	0	0	
Reduced v/c Ratio		1.11		1.06	0.44	0.08	1.14	0.99	0.10	0.50	0.98	

Intersection Summary Cycle Length: 125
Actuated Cycle Length: 125
Control Type: Actuated-Uncoordinated

Maximum v/c Ratio: 1.14
Intersection Signal Delay: 69.8

Intersection LOS: E Intersection Capacity Utilization 91.2% ICU Level of Service F

Analysis Period (min) 15

Volume exceeds capacity, queue is theoretically infinite.

Queue shown is maximum after two cycles.

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.



06/07/2018

Intersection Delay, s/veh	24.7						Ī
Intersection LOS	С						
Movement	EBT	EBR	WBL	WBT	NBL	NBR	
Lane Configurations	<u> </u>	7	******	4	*y*	HUIT	
Traffic Vol, veh/h	337	164	191	313	131	62	
Future Vol, veh/h	337	164	191	313	131	62	
Peak Hour Factor	0.88	0.88	0.88	0.88	0.88	0.88	
Heavy Vehicles, %	2	2	2	2	2	2	
Mymt Flow	383	186	217	356	149	70	
Number of Lanes	1	1	0	1	1	0	
Approach	EB		WB		NB		
Opposing Approach	WB		EB		140		
Opposing Lanes	1		2		0		
Conflicting Approach Left			NB		EB		
Conflicting Lanes Left	0		1		2		
Conflicting Approach Right	NB				WB		
Conflicting Lanes Right	1		0		W D		
HCM Control Delay	16		37.5		13.9		
HCM LOS	C		37.5		13.9 B		
TIGWI EU3	C		L		Б		
Lane		NBLn1	EBLn1				
Vol Left, %		68%	0%	0%	38%		
Vol Thru, %		0%	100%	0%	62%		
Vol Right, %		32%	0%	100%	0%		
Sign Control		Stop	Stop	Stop	Stop		
Traffic Vol by Lane		193	337	164	504		
LT Vol		131	0	0	191		
Through Vol		0	337	0	313		
RT Vol		62	0	164	0		
Lane Flow Rate		219	383	186	573		
Geometry Grp		2	7	7	5		
Degree of Util (X)		0.398	0.639	0.274	0.89		
Departure Headway (Hd)		6.538	6.008	5.297	5.594		
Convergence, Y/N		Yes	Yes	Yes	Yes		
Cap		548	601	677	647		
Service Time		4.601	3.761	3.049	3.639		
HCM Lane V/C Ratio		0.4	0.637	0.275	0.886		
HCM Control Delay		13.9	18.9	10.1	37.5		
HCM Lane LOS		В	С	В	Е		
HCM 95th-tile Q		1.9	4.5	1.1	10.9		

Intersection												
Int Delay, s/veh	4.3											
	EDI	FDT	EDD.	MDI	MOT	WDD	NIDI	NDT	NDD	CDI	CDT	CDD
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	ግ 25	↑1> 322	29	9 8	↑1 > 206	112	17	4	7	82	4	14
Traffic Vol, veh/h Future Vol, veh/h	25	322	29	88	206	112	17	0	7	82	0	14
Conflicting Peds, #/hr	25	322	29	00	200	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	1166	1166	None	-	1166	None	Siup	Siup	None	Stop -	Jiup -	None
Storage Length	100		NOTIC -	150		NOTIC -			NOTIC			IVOIIC
Veh in Median Storage		0		-	0			0			0	
Grade. %	. "	0			0			0			0	
Peak Hour Factor	83	83	83	83	83	83	83	83	83	83	83	83
Heavy Vehicles, %	2	2	2	2	2	2	2	2	2	2	2	2
Mymt Flow	30	388	35	106	248	135	20	0	8	99	0	17
Major/Minor N	Major1			Major2			Minor1		N	Minor2		
Conflicting Flow All	383	0	0	423	0	0	802	1061	212	782	1011	192
Stage 1	303	-	U	423	U	-	466	466	212	528	528	172
Stage 2	- 1						336	595		254	483	
Critical Hdwy	4.14			4.14			7.54	6.54	6.94	7.54	6.54	6.94
Critical Hdwy Stg 1	4.14						6.54	5.54	0.71	6.54	5.54	0.74
Critical Hdwy Stg 2		-					6.54	5.54		6.54	5.54	
Follow-up Hdwy	2.22			2.22			3.52	4.02	3.32	3.52	4.02	3.32
Pot Cap-1 Maneuver	1172	-		1133		-	275	222	793	284	238	817
Stage 1	-						546	561		502	526	
Stage 2	-	-	-	-	-		652	491	-	728	551	-
Platoon blocked, %												
Mov Cap-1 Maneuver	1172			1133	-		245	196	793	256	210	817
Mov Cap-2 Maneuver	-	-	-	-	-	-	245	196	-	256	210	-
Stage 1	-	-	-	-	-	-	532	546	-	489	477	-
Stage 2	-	-	-	-	-	-	579	445	-	702	537	-
Approach	EB			WB			NB			SB		
HCM Control Delay, s	0.5			1.8			17.9			26.1		
HCM LOS							С			D		
Minor Lane/Major Mvm	t	NBLn1	EBL	EBT	EBR	WBL	WBT	WRP	SBLn1			
Capacity (veh/h)		307	1172	LDI	LDIC	1133	WDI	WDIX.	284			
HCM Lane V/C Ratio		0.094	0.026		- 1	0.094			0.407			
HCM Control Delay (s)		17.9	8.2			8.5			26.1			
HCM Lane LOS		C	Α.2			Α.5			D			
HCM 95th %tile Q(veh)		0.3	0.1			0.3			1.9			
HOW JOHN JOHN Q(VEH)		0.3	0.1			0.3			1.7			

Through Vol

RT Vol Lane Flow Rate

Cap Service Time

HCM Lane V/C Ratio

HCM Control Delay HCM Lane LOS

HCM 95th-tile Q

Geometry Grp
Degree of Util (X)
Departure Headway (Hd)
Convergence, Y/N

Intersection												
Intersection Delay, s/veh	9.7											
Intersection LOS	Α											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	ሻ	↑ ₽		ሻ	ħβ			4			4	
Traffic Vol, veh/h	14	153	7	21	96	137	19	2	90	122	5	10
Future Vol, veh/h	14	153	7	21	96	137	19	2	90	122	5	10
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles, %	2	2	2	2	2	2	2	2	2	2	2	2
Mvmt Flow	15	166	8	23	104	149	21	2	98	133	5	11
Number of Lanes	1	2	0	1	2	0	0	1	0	0	1	0
Approach	EB			WB			NB			SB		
Opposing Approach	WB			EB			SB			NB		
Opposing Lanes	3			3			1			1		
Conflicting Approach Left	SB			NB			EB			WB		
Conflicting Lanes Left	1			1			3			3		
Conflicting Approach Right	NB			SB			WB			EB		
Conflicting Lanes Right	1			1			3			3		
HCM Control Delay	9.4			9.3			9.4			11.1		
HCM LOS	Α			Α			Α			В		

Lane	NBLn1	EBLn1	EBLn2	EBLn3	WBLn1	WBLn2	WBLn3	SBLn1
Vol Left, %	17%	100%	0%	0%	100%	0%	0%	89%
Vol Thru, %	2%	0%	100%	88%	0%	100%	19%	4%
Vol Right, %	81%	0%	0%	12%	0%	0%	81%	7%
Sign Control	Stop							
Traffic Vol by Lane	111	14	102	58	21	64	169	137
LT Vol	19	14	0	0	21	0	0	122
Through Vol	2	0	102	51	0	64	32	5
RT Vol	90	0	0	7	0	0	137	10
Lane Flow Rate	121	15	111	63	23	70	184	149
Geometry Grp	7	7	7	7	7	7	7	7
Degree of Util (X)	0.182	0.026	0.176	0.099	0.039	0.109	0.258	0.258
Departure Headway (Hd)	5.433	6.231	5.724	5.638	6.145	5.639	5.064	6.234
Convergence, Y/N	Yes							
Cap	661	577	630	639	586	639	713	578
Service Time	3.158	3.938	3.431	3.345	3.849	3.343	2.768	3.958
HCM Lane V/C Ratio	0.183	0.026	0.176	0.099	0.039	0.11	0.258	0.258
HCM Control Delay	9.4	9.1	9.7	9	9.1	9	9.5	11.1
HCM Lane LOS	Α	Α	Α	Α	Α	Α	Α	В
HCM 95th-tile Q	0.7	0.1	0.6	0.3	0.1	0.4	1	1

Intersection												
Intersection Delay, s/veh	9.3											
Intersection LOS	Α											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4						*	7	7	^	
Traffic Vol, veh/h	39	7	5	0	0	0	0	8	46	191	68	0
Future Vol, veh/h	39	7	5	0	0	0	0	8	46	191	68	0
Peak Hour Factor	0.82	0.82	0.82	0.82	0.82	0.82	0.82	0.82	0.82	0.82	0.82	0.82
Heavy Vehicles, %	2	2	2	2	2	2	2	2	2	2	2	2
Mvmt Flow	48	9	6	0	0	0	0	10	56	233	83	0
Number of Lanes	0	1	0	0	0	0	0	1	1	1	2	0
Approach	EB							NB		SB		
Opposing Approach								SB		NB		
Opposing Lanes	0							3		2		
Conflicting Approach Left	SB							EB				
Conflicting Lanes Left	3							1		0		
Conflicting Approach Right	NB									EB		
Conflicting Lanes Right	2							0		1		
HCM Control Delay	9.1							7.8		9.7		
HCM LOS	Α							Α		Α		
Lane		NBLn1	NBLn2	EBLn1	SBLn1	SBLn2	SBLn3					
Vol Left, %		0%	0%	76%	100%	0%	0%					
Vol Thru, %		100%	0%	14%	0%	100%	100%					
Vol Right, %		0%	100%	10%	0%	0%	0%					
Sign Control		Stop	Stop	Stop	Stop	Stop	Stop					
Traffic Vol by Lane		8	46	51	191	34	34					
LT Vol		0	0	39	191	0	0					
700				-								

62 233

627 3.452 2.986

> 9.1 10.7

0.3

5.354 4.651 5.729 5.286 Yes

771

0.015 0.073 0.099

0.2

0.015 0.072 0.099 0.342 0.055 0.035

Yes Yes

1.5

34 34

41 41

2.484 0.746

0.34 0.054 0.035

7.8 5.9

0.2

Yes

8

10 56

3.078 2.375

> 8.2 7.7

0

Intersection												
Intersection Delay, s/veh	10.7											
Intersection LOS	В											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		44			4		7	^			4	
Traffic Vol, veh/h	2	0	3	87	3	128	8	42	0	0	168	190
Future Vol, veh/h	2	0	3	87	3	128	8	42	0	0	168	190
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles, %	2	2	2	2	2	2	2	2	2	2	2	2
Vivmt Flow	2	0	3	97	3	142	9	47	0	0	187	211
Number of Lanes	0	1	0	0	1	0	1	1	0	0	1	0
Approach	EB			WB			NB				SB	
Opposing Approach	WB			EB			SB				NB	
Opposing Lanes	1			1			1				2	
Conflicting Approach Left	SB			NB			EB				WB	
Conflicting Lanes Left	1			2			1				1	
Conflicting Approach Right	NB			SB			WB				EB	
Conflicting Lanes Right	2			1			1				1	
HCM Control Delay	8.1			9.9			8.7				11.5	
HCM LOS	Α			Α			Α				В	
Lane		NBLn1	NBLn2	EBLn1	WBLn1	SBLn1						
/ol Left, %		100%	0%	40%	40%	0%						
/ol Thru, %		0%	100%	0%	1%	47%						
/ol Right, %		0%	0%	60%	59%	53%						
Sign Control		Stop	Stop	Stop	Stop	Stop						
raffic Vol by Lane		8	42	5	218	358						
_T Vol		8	0	2	87	0						
Through Vol		0	42	0	3	168						
RT Vol		0	0	3	128	190						
ane Flow Rate		9	47	6	242	398						
Geometry Grp		7	7	2	2	5						
		0.015	0.071	0.008	0.316	0.485						
Departure Headway (Hd)		5.993	5.488	5.013	4.696	4.392						
Departure Headway (Hd) Convergence, Y/N		5.993 Yes	5.488 Yes	5.013 Yes	4.696 Yes	Yes						
Departure Headway (Hd) Convergence, Y/N Cap		5.993 Yes 595	5.488 Yes 650	5.013 Yes 708	4.696 Yes 763	Yes 819						
Departure Headway (Hd) Convergence, Y/N Cap Service Time		5.993 Yes 595 3.754	5.488 Yes 650 3.248	5.013 Yes 708 3.085	4.696 Yes 763 2.745	Yes 819 2.434						
Departure Headway (Hd) Convergence, Y/N Cap Service Time HCM Lane V/C Ratio		5.993 Yes 595 3.754 0.015	5.488 Yes 650 3.248 0.072	5.013 Yes 708 3.085 0.008	4.696 Yes 763 2.745 0.317	Yes 819 2.434 0.486						
Degree of Util (X) Departure Headway (Hd) Convergence, Y/N Cap Service Time HCM Lane V/C Ratio HCM Control Delay		5.993 Yes 595 3.754 0.015 8.8	5.488 Yes 650 3.248 0.072 8.7	5.013 Yes 708 3.085 0.008 8.1	4.696 Yes 763 2.745 0.317 9.9	Yes 819 2.434 0.486 11.5						
Departure Headway (Hd) Convergence, Y/N Cap Service Time HCM Lane V/C Ratio HCM Control Delay HCM Lane LOS		5.993 Yes 595 3.754 0.015 8.8 A	5.488 Yes 650 3.248 0.072 8.7 A	5.013 Yes 708 3.085 0.008 8.1 A	4.696 Yes 763 2.745 0.317 9.9 A	Yes 819 2.434 0.486 11.5 B						
Departure Headway (Hd) Convergence, Y/N Cap Service Time HCM Lane V/C Ratio HCM Control Delay		5.993 Yes 595 3.754 0.015 8.8	5.488 Yes 650 3.248 0.072 8.7	5.013 Yes 708 3.085 0.008 8.1	4.696 Yes 763 2.745 0.317 9.9	Yes 819 2.434 0.486 11.5						

Intersection						
Int Delay, s/veh	0.9					
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	W			A	<u>551</u>	ODIN
Traffic Vol, veh/h	17	28	0	172	330	0
Future Vol. veh/h	17	28	0	172	330	0
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None		None	-	None
Storage Length	0	-	-	-		
Veh in Median Storage	e, # 0			0	0	
Grade, %	0	-		0	0	
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	18	30	0	187	359	0
Major/Minor	Minor2		Major1		Aniora	
Major/Minor Conflicting Flow All	546	359	viajui i -	0	Major2	0
Stage 1	359	339		U		U
Stage 2	187					
Critical Hdwy	6.42	6.22				
	5.42	0.22				- 1
Critical Hdwy Stg 1 Critical Hdwy Stg 2	5.42	-		-		
Follow-up Hdwy	3.518	3.318				
Pot Cap-1 Maneuver	499	685	0		- 1	0
	707	080			- 1	0
Stage 1 Stage 2	845		0	-		0
	843	-	U			U
Platoon blocked, %	400	/05			-	
Mov Cap-1 Maneuver	499	685	-	-		
Mov Cap-2 Maneuver	499	-	-	-	-	
Stage 1	707	-				-
Stage 2	845		-		-	
Approach	EB		NB		SB	
HCM Control Delay, s	11.5		0		0	
HCM LOS	В					
Mineral and Marie Man		NDT	EDI1	CDT		
Minor Lane/Major Mvn	11	INRT	EBLn1	SBT		
Capacity (veh/h)		-	600	-		
HCM Cantral Dalay (a)			0.082			
HCM Control Delay (s)			11.5			
HCM Lane LOS	١	-	В	-		
HCM 95th %tile Q(veh)		0.3	-		

Loveless Tract TIA Full Build Alternative (2021) 1: FM 2818 & F and B Road/HSC Parkway

Juu, i	1001	antivay
	AM	Peak Hour

	•	-	\rightarrow	•	-	*	4	†	1	-	↓	4
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4		ሻ	↑	7	7	^	7	ሻ	↑ ↑	
Traffic Volume (vph)	74	44	53	78	57	19	196	801	202	207	935	182
Future Volume (vph)	74	44	53	78	57	19	196	801	202	207	935	182
Satd. Flow (prot)	0	1747	0	1770	1863	1583	1770	3539	1583	1770	3454	0
Flt Permitted		0.979		0.950			0.078			0.217		
Satd. Flow (perm)	0	1747	0	1770	1863	1583	145	3539	1583	404	3454	0
Satd. Flow (RTOR)		15				236			236		23	
Lane Group Flow (vph)	0	199	0	91	66	22	228	931	235	241	1299	0
Turn Type	Split	NA		Split	NA	Free	pm+pt	NA	Free	pm+pt	NA	
Protected Phases	3	3		4	4		1	6		5	2	
Permitted Phases						Free	6		Free	2		
Total Split (s)	23.0	23.0		16.0	16.0		24.0	60.0		26.0	62.0	
Total Lost Time (s)		7.0		7.0	7.0		8.0	8.0		8.0	8.0	
Act Effct Green (s)		15.3		8.7	8.7	122.4	69.5	55.2	122.4	67.2	54.1	
Actuated g/C Ratio		0.12		0.07	0.07	1.00	0.57	0.45	1.00	0.55	0.44	
v/c Ratio		0.86		0.72	0.50	0.01	0.84	0.58	0.15	0.65	0.84	
Control Delay		81.6		87.2	69.1	0.0	56.3	27.5	0.2	20.2	36.7	
Queue Delay		0.0		0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Total Delay		81.6		87.2	69.1	0.0	56.3	27.5	0.2	20.2	36.7	
LOS		F		F	E	Α	E	С	Α	С	D	
Approach Delay		81.6			69.8			27.6			34.1	
Approach LOS		F			Е			С			С	
Queue Length 50th (ft)		148		73	52	0	121	285	0	80	485	
Queue Length 95th (ft)		#260		#147	97	0	#228	356	0	113	543	
Internal Link Dist (ft)		2234			595			1612			403	
Turn Bay Length (ft)				125		300	200		500	200		
Base Capacity (vph)		241		130	137	1583	296	1597	1583	438	1538	
Starvation Cap Reductn		0		0	0	0	0	0	0	0	0	
Spillback Cap Reductn		0		0	0	0	0	0	0	0	0	
Storage Cap Reductn		0		0	0	0	0	0	0	0	0	
Reduced v/c Ratio		0.83		0.70	0.48	0.01	0.77	0.58	0.15	0.55	0.84	

Intersection Summary

Cycle Length: 125
Actuated Cycle Length: 122.4
Control Type: Actuated-Uncoordinated

Maximum v/c Ratio: 0.86 Intersection Signal Delay: 36.1
Intersection Capacity Utilization 78.0%

Intersection LOS: D ICU Level of Service D

Analysis Period (min) 15

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

Splits and Phases: 1: FM 2818 & F and B Road/HSC Parkway



Synchro 10 Report 06/07/2018

Loveless Tract TIA Full Build Alternative (2021) 2: Turkey Creek Road & HSC Parkway/F and B Road AM Peak Hour

Intersection						
Intersection Delay, s/veh	12.8					
Intersection LOS	В					
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	<u></u>	LDIK	WDL	सी	NDL Y	NDIX
Traffic Vol, veh/h	133	1 91	78	324	'T' 81	60
Future Vol, veh/h	133	91	78	324	81	60
Peak Hour Factor	0.86	0.86	0.86	0.86	0.86	0.86
Heavy Vehicles, %	0.60	0.00	2	0.00	0.00	0.00
Mymt Flow	155	106	91	377	94	70
Number of Lanes	100	100	0	1	1	0
Number of Lanes			U			U
Approach	EB		WB		NB	
Opposing Approach	WB		EB			
Opposing Lanes	1		2		0	
Conflicting Approach Left			NB		EB	
Conflicting Lanes Left	0		1		2	
Conflicting Approach Right	NB				WB	
Conflicting Lanes Right	1		0		1	
HCM Control Delay	9.2		15.7		10.3	
HCM LOS	Α		С		В	
Lano		NIDI n1	ERI n1	ERI n2	WRI n1	
Lane Vol Loft 9/		NBLn1	EBLn1	EBLn2	WBLn1	
Vol Left, %		57%	0%	0%	19%	
Vol Left, % Vol Thru, %		57% 0%	0% 100%	0% 0%	19% 81%	
Vol Left, % Vol Thru, % Vol Right, %		57% 0% 43%	0% 100% 0%	0% 0% 100%	19% 81% 0%	
Vol Left, % Vol Thru, % Vol Right, % Sign Control		57% 0% 43% Stop	0% 100% 0% Stop	0% 0% 100% Stop	19% 81% 0% Stop	
Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane		57% 0% 43% Stop 141	0% 100% 0% Stop 133	0% 0% 100% Stop 91	19% 81% 0% Stop 402	
Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol		57% 0% 43% Stop 141 81	0% 100% 0% Stop 133	0% 0% 100% Stop 91	19% 81% 0% Stop 402 78	
Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol		57% 0% 43% Stop 141 81 0	0% 100% 0% Stop 133 0	0% 0% 100% Stop 91 0	19% 81% 0% Stop 402 78 324	
Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Tri Vol RT Vol		57% 0% 43% Stop 141 81 0	0% 100% 0% Stop 133 0 133	0% 0% 100% Stop 91 0 0	19% 81% 0% Stop 402 78 324	
Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate		57% 0% 43% Stop 141 81 0 60	0% 100% 0% Stop 133 0 133 0	0% 0% 100% Stop 91 0 0 91 106	19% 81% 0% Stop 402 78 324 0	
Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate Geometry Grp		57% 0% 43% Stop 141 81 0 60 164	0% 100% 0% Stop 133 0 133 0 155	0% 0% 100% Stop 91 0 0 91 106	19% 81% 0% Stop 402 78 324 0 467	
Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate Geometry Grp Degree of Util (X)		57% 0% 43% Stop 141 81 0 60 164 2	0% 100% 0% Stop 133 0 133 0 155 7	0% 0% 100% Stop 91 0 91 106 7	19% 81% 0% Stop 402 78 324 0 467 5	
Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate Geometry Grp Degree of Util (X) Departure Headway (Hd)		57% 0% 43% Stop 141 81 0 60 164 2 0.247 5.431	0% 100% 0% Stop 133 0 133 0 155 7 0.232 5.399	0% 0% 100% Stop 91 0 91 106 7 0.138 4.692	19% 81% 0% Stop 402 78 324 0 467 5 0.625 4.817	
Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate Geometry Grp Degree of Util (X) Departure Headway (Hd) Convergence, Y/N		57% 0% 43% Stop 141 81 0 60 164 2 0.247 5.431 Yes	0% 100% 0% Stop 133 0 133 0 155 7 0.232 5.399	0% 0% 100% Stop 91 0 91 106 7 0.138 4.692 Yes	19% 81% 0% Stop 402 78 324 0 467 5 0.625 4.817 Yes	
Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate Geometry Grp Degree of Util (X) Departure Headway (Hd) Convergence, Y/N Cap		57% 0% 43% Stop 141 81 0 60 164 2 0.247 5.431 Yes 656	0% 100% 0% Stop 133 0 133 0 155 7 0.232 5.399 Yes 660	0% 0% 100% Stop 91 0 0 91 106 7 0.138 4.692 Yes	19% 81% 0% Stop 402 78 324 0 467 5 0.625 4.817 Yes 743	
Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate Geometry Grp Degree of Util (X) Departure Headway (Hd) Convergence, Y/N Cap Service Time		57% 0% 43% Stop 141 81 0 60 164 2 0.247 5.431 Yes 656 3.516	0% 100% 0% Stop 133 0 133 0 155 7 0.232 5.399 Yes 660 3.173	0% 0% 100% Stop 91 0 0 91 106 7 0.138 4.692 Yes 757 2.465	19% 81% 0% Stop 402 78 324 0 467 5 0.625 4.817 Yes 743 2.879	
Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate Geometry Grp Degree of Util (X) Departure Headway (Hd) Convergence, Y/N Cap Service Time HCM Lane V/C Ratio		57% 0% 43% Stop 141 81 0 60 164 2 0.247 5.431 Yes 656 3.516 0.25	0% 100% 0% Stop 133 0 133 0 155 7 0.232 5.399 Yes 660 3.173 0.235	0% 0% 100% Stop 91 0 0 91 106 7 0.138 4.692 Yes 757 2.465	19% 81% 0% Stop 402 78 324 0 467 5 0.625 4.817 Yes 743 2.879 0.629	
Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate Geometry Grp Degree of Util (X) Departure Headway (Hd) Convergence, Y/N Cap Service Time HCM Lane V/C Ratio HCM Control Delay		57% 0% 43% Stop 141 81 0 60 164 2 0.247 5.431 Yes 656 3.516 0.25 10.3	0% 100% 0% Stop 133 0 155 7 0.232 5.399 Yes 660 3.173 0.235 9.8	0% 0% 100% Stop 91 0 0 91 106 7 0.138 4.692 Yes 757 2.465 0.14	19% 81% 0% Stop 402 78 324 0 467 5 0.625 4.817 Yes 743 2.879 0.629 15.7	
Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate Geometry Grp Degree of Util (X) Departure Headway (Hd) Convergence, Y/N Cap Service Time HCM Lane V/C Ratio		57% 0% 43% Stop 141 81 0 60 164 2 0.247 5.431 Yes 656 3.516 0.25	0% 100% 0% Stop 133 0 133 0 155 7 0.232 5.399 Yes 660 3.173 0.235	0% 0% 100% Stop 91 0 0 91 106 7 0.138 4.692 Yes 757 2.465	19% 81% 0% Stop 402 78 324 0 467 5 0.625 4.817 Yes 743 2.879 0.629	

Synchro 10 Report 06/07/2018

Intersection												
Int Delay, s/veh	3.9											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	7	∳ β			413			4			44	
Traffic Vol. veh/h	5	130	9	2	405	36	30	0	6	116	0	24
Future Vol. veh/h	5	130	9	2	405	36	30	0	6	116	0	24
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-		None	-		None	-	-	None	-	-	None
Storage Length	100		-	-		-			-	-		
Veh in Median Storage,	# -	0	-	-	0	-		0	-	-	0	
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	89	89	89	89	89	89	89	89	89	89	89	89
Heavy Vehicles, %	2	2	2	2	2	2	2	2	2	2	2	2
Mvmt Flow	6	146	10	2	455	40	34	0	7	130	0	27
	ajor1			Major2			/linor1			/linor2		
Conflicting Flow All	495	0	0	156	0	0	395	662	78	564	647	248
Stage 1	-		-	-		-	163	163	-	479	479	
Stage 2	-	-	-	-	-	-	232	499	-	85	168	-
Critical Hdwy	4.14		-	4.14		-	7.54	6.54	6.94	7.54	6.54	6.94
Critical Hdwy Stg 1	-	-	-	-	-	-	6.54	5.54	-	6.54	5.54	-
Critical Hdwy Stg 2	-		-	-	-	-	6.54	5.54	-	6.54	5.54	-
Follow-up Hdwy	2.22	-	-	2.22	-	-	3.52	4.02	3.32	3.52	4.02	3.32
	1065	-	-	1422	-	-	539	381	967	408	388	752
Stage 1	-	-	-	-	-	-	823	762	-	537	553	-
Stage 2	-		-	-	-	-	750	542	-	913	758	
Platoon blocked, %		-	-		-	-						
	1065	-	-	1422	-	-	517	378	967	403	385	752
Mov Cap-2 Maneuver	-	-	-	-	-	-	517	378	-	403	385	-
Stage 1	-	-	-	-	-	-	818	757		534	552	-
Stage 2	-	-		-	-	-	722	541	-	902	753	-
				1110								
Approach	EB			WB			NB			SB		
HCM Control Delay, s	0.3			0			11.9			17.8		
HCM LOS							В			С		
Minor Lane/Major Mvmt		VBI n1	EBL	EBT	EBR	WBL	WBT	WBR:	CDI n1			
		560			EBR	1422			438			
Capacity (veh/h)			1065	-	-		-	-				
HCM Control Doloy (c)		0.072		-	-	0.002	-		0.359			
HCM Long LOS		11.9	8.4	-	-	7.5	0	-	17.8			
HCM Lane LOS		0.2	A 0	-	-	A 0	Α		C 1.6			
HCM 95th %tile Q(veh)		0.2	U	-	-	U	-	-	1.0			

Intersection												
Intersection Delay, s/veh	9.2											
Intersection LOS	A											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBF
Lane Configurations	*	† 1>		*	ΦÞ			4			4	
Traffic Vol, veh/h	0	59	10	98	248	94	2	0	18	62	5	18
Future Vol. veh/h	0	59	10	98	248	94	2	0	18	62	5	18
Peak Hour Factor	0.87	0.87	0.87	0.87	0.87	0.87	0.87	0.87	0.87	0.87	0.87	0.87
Heavy Vehicles, %	2	2	2	2	2	2	2	2	2	2	2	2
Mvmt Flow	0	68	11	113	285	108	2	0	21	71	6	21
Number of Lanes	1	2	0	1	2	0	0	1	0	0	1	(
Approach	EB			WB			NB			SB		
Opposing Approach	WB			EB			SB			NB		
Opposing Lanes	3			3			1			1		
Conflicting Approach Left	SB			NB			EB			WB		
Conflicting Lanes Left	1			1			3			3		
Conflicting Approach Right	NB			SB			WB			EB		
Conflicting Lanes Right	1			1			3			3		
HCM Control Delay	8.4			9.2			8.3			10		
HCM LOS	Α			Α			Α			Α		
Lane		NBLn1	EBLn1	EBLn2	EBLn3	WBLn1	WBLn2		SBLn1			
Vol Left, %		10%	0%	0%	0%	100%	0%	0%	73%			
Vol Thru, %		0%	100%	100%	66%	0%	100%	47%	6%			
Vol Right, %		90%	0%	0%	34%	0%	0%	53%	21%			
Sign Control		Stop	Stop	Stop	Stop	Stop	Stop	Stop	Stop			
Traffic Vol by Lane		20	0	39	30	98	165	177	85			
LT Vol		2	0	0	0	98	0	0	62			
Through Vol		0	0	39	20	0	165	83	5			
RT Vol		18	0	0 45	10	0	0	94	18			
Lane Flow Rate		23 7	7	45 7	34	113	190	203	98			
Geometry Grp					7	7	7	7	7			
Degree of Util (X)		0.034	5.37	0.067 5.37	0.049	0.171	0.262	0.259	0.164 6.028			
Departure Headway (Hd)		5.35 Yes	5.37 Yes	Yes	5.133 Yes	5.471 Yes	4.969	4.595	6.028 Yes			
Convergence, Y/N							Yes 722	Yes	594			
Cap Service Time		666 3.108	3.121	665 3.121	695 2.884	656 3.205	2.703	782 2.328	3,777			
		0.035	3.121	0.068		0.172	0.263	0.26	0.165			
HCM Control Dolay		8.3	8.1	8.5	0.049	9.3	9.5	8.9	10			
HCM Control Delay HCM Lane LOS		8.3 A	8.1 N	8.5 A	8.2 A	9.3 A	9.5 A	8.9 A	10 A			
HCM 95th-tile Q		0.1	0	0.2	0.2	0.6	1	1	0.6			
FIGINI FORT-UIR Q		U. I	U	0.2	0.2	0.0	- 1		U.0			

Intersection												
Intersection Delay, s/veh	9.7											
Intersection LOS	Α											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		43-						*	7	ħ	^	
Traffic Vol, veh/h	42	4	6	0	0	0	0	4	16	174	30	0
Future Vol, veh/h	42	4	6	0	0	0	0	4	16	174	30	0
Peak Hour Factor	0.75	0.75	0.75	0.75	0.75	0.75	0.75	0.75	0.75	0.75	0.75	0.75
Heavy Vehicles %	2	2	2	2	2	2	2	2	2	2	2	2

Heavy Vehicles, %	2	2	2	2	2	2	2	2	2	2	2	2
Mvmt Flow	56	5	8	0	0	0	0	5	21	232	40	0
Number of Lanes	0	1	0	0	0	0	0	1	1	1	2	0
Approach	EB							NB		SB		
Opposing Approach								SB		NB		
Opposing Lanes	0							3		2		
Conflicting Approach Left	SB							EB				
Conflicting Lanes Left	3							1		0		
Conflicting Approach Right	NB									EB		
Conflicting Lanes Right	2							0		1		
HCM Control Delay	9							7.6		10.1		
HCM LOS	Α							Α		В		

Lane	NBLn1	NBLn2	EBLn1	SBLn1	SBLn2	SBLn3
Vol Left, %	0%	0%	81%	100%	0%	0%
Vol Thru, %	100%	0%	8%	0%	100%	100%
Vol Right, %	0%	100%	12%	0%	0%	0%
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop
Traffic Vol by Lane	4	16	52	174	15	15
LT Vol	0	0	42	174	0	0
Through Vol	4	0	4	0	15	15
RT Vol	0	16	6	0	0	0
Lane Flow Rate	5	21	69	232	20	20
Geometry Grp	8	8	7	7	7	7
Degree of Util (X)	0.008	0.027	0.108	0.34	0.026	0.016
Departure Headway (Hd)	5.314	4.612	5.588	5.273	4.672	2.935
Convergence, Y/N	Yes	Yes	Yes	Yes	Yes	Yes
Cap	675	777	643	687	755	1187
Service Time	3.036	2.333	3.304	2.973	2.471	0.733
HCM Lane V/C Ratio	0.007	0.027	0.107	0.338	0.026	0.017
HCM Control Delay	8.1	7.5	9	10.7	7.6	5.8
HCM Lane LOS	Α	Α	Α	В	Α	Α
HCM 95th-tile Q	0	0.1	0.4	1.5	0.1	0

Intersection												
Intersection Delay, s/veh	9.1											
Intersection LOS	Α											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4		7	†			4	
Traffic Vol, veh/h	3	0	6	23	9	100	21	26	0	0	166	64
Future Vol, veh/h	3	0	6	23	9	100	21	26	0	0	166	64
Peak Hour Factor	0.79	0.79	0.79	0.79	0.79	0.79	0.79	0.79	0.79	0.79	0.79	0.79
Heavy Vehicles, %	2	2	2	2	2	2	2	2	2	2	2	2
Mvmt Flow	4	0	8	29	11	127	27	33	0	0	210	81
Number of Lanes	0	1	0	0	1	0	1	1	0	0	1	0
Approach	EB			WB			NB				SB	
Opposing Approach	WB			EB			SB				NB	
Opposing Lanes	1			1			1				2	
Conflicting Approach Left	SB			NB			EB				WB	
Conflicting Lanes Left	1			2			1				1	
Conflicting Approach Right	NB			SB			WB				EB	
Conflicting Lanes Right	2			1			1				1	
HCM Control Delay	7.7			8.5			8.4				9.7	
HCM LOS	Α			Α			Α				Α	

Lane	NBLn1	NBLn2	EBLn1	WBLn1	SBLn1
Vol Left, %	100%	0%	33%	17%	0%
Vol Thru, %	0%	100%	0%	7%	72%
Vol Right, %	0%	0%	67%	76%	28%
Sign Control	Stop	Stop	Stop	Stop	Stop
Traffic Vol by Lane	21	26	9	132	230
LT Vol	21	0	3	23	0
Through Vol	0	26	0	9	166
RT Vol	0	0	6	100	64
Lane Flow Rate	27	33	11	167	291
Geometry Grp	7	7	2	2	5
Degree of Util (X)	0.042	0.047	0.015	0.201	0.352
Departure Headway (Hd)	5.676	5.172	4.608	4.335	4.347
Convergence, Y/N	Yes	Yes	Yes	Yes	Yes
Cap	631	692	776	829	827
Service Time	3.407	2.903	2.641	2.358	2.371
HCM Lane V/C Ratio	0.043	0.048	0.014	0.201	0.352
HCM Control Delay	8.7	8.2	7.7	8.5	9.7
HCM Lane LOS	Α	Α	Α	Α	Α
HCM 95th-tile Q	0.1	0.1	0	0.7	1.6

Loveless Tract TIA Full Build Alternative (2021) 7: Turkey Creek Road & Proposed Driveway AM Peak Hour

Intersection						
Int Delay, s/veh	2.4					
		EDD	ND:	NDT	007	CDE
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	Y	F.2	4.	4	₽ 170	
Traffic Vol, veh/h	30	53	16	115	178	9
Future Vol, veh/h	30	53	16	115	178	9
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage		-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	33	58	17	125	193	10
Major/Minor	Minor2		Major1		Anior2	
			Major1		Najor2	
Conflicting Flow All	357	198	203	0	-	0
Stage 1	198		-	-	-	-
Stage 2	159	-		-	-	-
Critical Hdwy	6.42	6.22	4.12	-	-	-
Critical Hdwy Stg 1	5.42	-	-	-	-	-
Critical Hdwy Stg 2	5.42	-	-	-	-	-
Follow-up Hdwy	3.518			-	-	-
Pot Cap-1 Maneuver	641	843	1369	-	-	-
Stage 1	835	-	-	-	-	-
Stage 2	870		-		-	-
Platoon blocked, %				-	-	-
Mov Cap-1 Maneuver	633	843	1369		-	-
Mov Cap-2 Maneuver	633	-	-	-	-	-
Stage 1	824				-	
Stage 2	870					
Olugo 2	3.0					
Approach	EB		NB		SB	
HCM Control Delay, s	10.4		0.9		0	
HCM LOS	В					
Minor Lana/Major Mum	nt.	NBL	NIDT	EBLn1	SBT	SBR
Minor Lane/Major Mvm	IL					
Capacity (veh/h)		1369		753	-	-
HCM Lane V/C Ratio		0.013	-	0.12	-	-
HCM Control Delay (s)		7.7	0	10.4	-	-
HCM Lane LOS		Α	Α	В	-	-
HCM 95th %tile Q(veh)	0	-	0.4	-	-

Synchro 10 Report 06/07/2018

Loveless Tract TIA Full Build Alternative (2021) 1: FM 2818 & F&B Road PM Peak Hour

	*	→	*	•	←	*	1	†	1	1	ļ	1
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4		ሻ	*	7	ሻ	^	7	ሻ	↑ ↑	
Traffic Volume (vph)	157	116	140	193	85	120	193	1324	140	52	912	192
Future Volume (vph)	157	116	140	193	85	120	193	1324	140	52	912	192
Satd. Flow (prot)	0	1743	0	1770	1863	1583	1770	3539	1583	1770	3447	0
Flt Permitted		0.981		0.950			0.078			0.088		
Satd. Flow (perm)	0	1743	0	1770	1863	1583	145	3539	1583	164	3447	0
Satd. Flow (RTOR)		19				236			236		22	
Lane Group Flow (vph)	0	449	0	210	92	130	210	1439	152	57	1200	0
Turn Type	Split	NA		Split	NA	Free	pm+pt	NA	Free	pm+pt	NA	
Protected Phases	3	3		4	4		1	6		5	2	
Permitted Phases						Free	6		Free	2		
Total Split (s)	35.0	35.0		21.0	21.0		17.0	57.0		12.0	52.0	
Total Lost Time (s)		7.0		7.0	7.0		8.0	8.0		8.0	8.0	
Act Effct Green (s)		28.0		14.0	14.0	125.0	58.6	51.4	125.0	48.0	44.0	
Actuated g/C Ratio		0.22		0.11	0.11	1.00	0.47	0.41	1.00	0.38	0.35	
v/c Ratio		1.11		1.06	0.44	0.08	1.14	0.99	0.10	0.50	0.98	
Control Delay		120.5		133.5	59.2	0.1	137.9	58.3	0.1	33.8	60.4	
Queue Delay		0.0		0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Total Delay		120.5		133.5	59.2	0.1	137.9	58.3	0.1	33.8	60.4	
LOS		F		F	E	Α	F	Е	Α	С	E	
Approach Delay		120.5			77.5			62.7			59.2	
Approach LOS		F			Е			Е			Е	
Queue Length 50th (ft)		~403		~186	71	0	~150	~659	0	24	494	
Queue Length 95th (ft)		#613		#345	127	0	#310	#799	0	48	#651	
Internal Link Dist (ft)		2234			595			1612			403	
Turn Bay Length (ft)				125		300	200		500	200		
Base Capacity (vph)		405		198	208	1583	185	1455	1583	114	1227	
Starvation Cap Reductn		0		0	0	0	0	0	0	0	0	
Spillback Cap Reductn		0		0	0	0	0	0	0	0	0	
Storage Cap Reductn		0		0	0	0	0	0	0	0	0	
Reduced v/c Ratio		1.11		1.06	0.44	0.08	1.14	0.99	0.10	0.50	0.98	
latana attan Communica												

Cycle Length: 125
Actuated Cycle Length: 125
Control Type: Actuated-Uncoordinated Maximum v/c Ratio: 1.14
Intersection Signal Delay: 69.8 Intersection Capacity Utilization 91.2%

Intersection LOS: E ICU Level of Service F

Analysis Period (min) 15

Volume exceeds capacity, queue is theoretically infinite.

Queue shown is maximum after two cycles.

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.



06/07/2018

Intersection Delay, s/veh	21.2						
Intersection LOS	C						
IIICI3CCIOII EO3	C						
Movement	EBT	EBR	WBL	WBT	NBL	NBR	
Lane Configurations	↑	7		ની	Y		
Traffic Vol, veh/h	337	161	220	284	78	62	
Future Vol, veh/h	337	161	220	284	78	62	
Peak Hour Factor	0.88	0.88	0.88	0.88	0.88	0.88	
Heavy Vehicles, %	2	2	2	2	2	2	
Mvmt Flow	383	183	250	323	89	70	
Number of Lanes	1	1	0	1	1	0	
Annragah	ED		WB		NB		
Approach	EB				IND		
Opposing Approach	WB		EB		^		
Opposing Lanes	1		2		0		
Conflicting Approach Left			NB		EB		
Conflicting Lanes Left	0		1		2		
Conflicting Approach Right	NB				WB		
Conflicting Lanes Right	1		0		1		
HCM Control Delay	14.4		30.6		11.9		
HCM LOS	В		D		В		
Lano	_	NRI n1	EBI n1	EBI n2	WRI n1		
Lane		NBLn1	EBLn1	EBLn2			
Vol Left, %		56%	0%	0%	44%		
Vol Left, % Vol Thru, %		56% 0%	0% 100%	0% 0%	44% 56%		
Vol Left, % Vol Thru, % Vol Right, %		56% 0% 44%	0% 100% 0%	0% 0% 100%	44% 56% 0%		
Vol Left, % Vol Thru, % Vol Right, % Sign Control		56% 0% 44% Stop	0% 100% 0% Stop	0% 0% 100% Stop	44% 56% 0% Stop		
Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane		56% 0% 44% Stop 140	0% 100% 0% Stop 337	0% 0% 100% Stop 161	44% 56% 0% Stop 504		
Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol		56% 0% 44% Stop 140 78	0% 100% 0% Stop 337	0% 0% 100% Stop 161	44% 56% 0% Stop 504 220		
Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol		56% 0% 44% Stop 140 78 0	0% 100% 0% Stop 337 0	0% 0% 100% Stop 161 0	44% 56% 0% Stop 504 220 284		
Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol		56% 0% 44% Stop 140 78 0	0% 100% 0% Stop 337 0 337	0% 0% 100% Stop 161 0	44% 56% 0% Stop 504 220 284		
Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol		56% 0% 44% Stop 140 78 0	0% 100% 0% Stop 337 0	0% 0% 100% Stop 161 0	44% 56% 0% Stop 504 220 284		
Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol		56% 0% 44% Stop 140 78 0	0% 100% 0% Stop 337 0 337	0% 0% 100% Stop 161 0	44% 56% 0% Stop 504 220 284		
Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate		56% 0% 44% Stop 140 78 0 62 159	0% 100% 0% Stop 337 0 337 0	0% 0% 100% Stop 161 0 0 161 183	44% 56% 0% Stop 504 220 284 0 573		
Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate Geometry Grp		56% 0% 44% Stop 140 78 0 62 159	0% 100% 0% Stop 337 0 337 0 383	0% 0% 100% Stop 161 0 0 161 183	44% 56% 0% Stop 504 220 284 0 573		
Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate Geometry Grp Degree of Util (X) Departure Headway (Hd)		56% 0% 44% Stop 140 78 0 62 159 2	0% 100% 0% Stop 337 0 337 0 383 7	0% 0% 100% Stop 161 0 0 161 183 7	44% 56% 0% Stop 504 220 284 0 573 5		
Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate Geometry Grp Degree of Util (X) Departure Headway (Hd) Convergence, Y/N		56% 0% 44% Stop 140 78 0 62 159 2 0.28 6.346	0% 100% 0% Stop 337 0 337 0 383 7 0.605 5.686	0% 0% 100% Stop 161 0 0 161 183 7 0.253 4.977	44% 56% 0% Stop 504 220 284 0 573 5 0.846 5.317		
Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate Geometry Grp Degree of Util (X) Departure Headway (Hd)		56% 0% 44% Stop 140 78 0 62 159 2 0.28 6.346 Yes	0% 100% 0% Stop 337 0 337 0 383 7 0.605 5.686 Yes	0% 0% 100% Stop 161 0 0 161 183 7 0.253 4.977 Yes	44% 56% 0% Stop 504 220 284 0 573 5 0.846 5.317 Yes		
Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate Geometry Grp Degree of Util (X) Departure Headway (Hd) Convergence, Y/N Cap Service Time		56% 0% 44% Stop 140 78 0 62 159 2 0.28 6.346 Yes 566 4.391	0% 100% 0% Stop 337 0 337 0 383 7 0.605 5.686 Yes 636 3.418	0% 0% 100% Stop 161 0 0 161 183 7 0.253 4.977 Yes 721 2.708	44% 56% 0% Stop 504 220 284 0 573 5 0.846 5.317 Yes 682 3.346		
Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate Geometry Grp Degree of Util (X) Departure Headway (Hd) Convergence, Y/N Cap Service Time HCM Lane V/C Ratio		56% 0% 44% Stop 140 78 0 62 159 2 0.28 6.346 Yes 566 4.391 0.281	0% 100% 0% Stop 337 0 337 0 0.605 5.686 Yes 636 3.418 0.602	0% 0% 100% Stop 161 0 0 161 183 7 0.253 4.977 Yes 721 2.708 0.254	44% 56% 0% Stop 504 220 284 0 573 5 0.846 5.317 Yes 682 3.346 0.84		
Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate Geometry Grp Degree of Util (X) Departure Headway (Hd) Convergence, Y/N Cap Service Time HCM Lane V/C Ratio HCM Control Delay		56% 0% 44% Stop 140 78 0 62 159 2 0.28 6.346 Yes 566 4.391 0.281 11.9	0% 100% 0% Stop 337 0 337 0 383 7 0.605 5.686 Yes 636 3.418 0.602 16.8	0% 0% 100% Stop 161 0 0 161 183 7 0.253 4.977 Yes 721 2.708 0.254 9.4	44% 56% 0% Stop 504 220 284 0 573 5 0.846 5.317 Yes 682 3.346 0.84 30.6		
Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate Geometry Grp Degree of Util (X) Departure Headway (Hd) Convergence, Y/N Cap Service Time HCM Lane V/C Ratio		56% 0% 44% Stop 140 78 0 62 159 2 0.28 6.346 Yes 566 4.391 0.281	0% 100% 0% Stop 337 0 337 0 0.605 5.686 Yes 636 3.418 0.602	0% 0% 100% Stop 161 0 0 161 183 7 0.253 4.977 Yes 721 2.708 0.254	44% 56% 0% Stop 504 220 284 0 573 5 0.846 5.317 Yes 682 3.346 0.84		

Intersection												
Int Delay, s/veh	2.7											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	EDL		EDR	WDL		WDK	INDL	4	NDK	SDL	3B1 ∰	JDR
Traffic Vol, veh/h	1 25	↑↑ 322	29	1	↑1 → 206	112	17	0	3	82	↔	14
		322	29		206	112	17	0	3	82		14
Future Vol, veh/h Conflicting Peds, #/hr	25 0	322	29	6	206	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	riee	Free -	None	Free	riee -	None	Stob	Stop	None	Stop -	Stop	None
Storage Length	100		None	150		None			None			None
Veh in Median Storage,		0		130	0			0	_		0	
Grade. %	# -	0			0			0			0	
Peak Hour Factor	83	83	83	83	83	83	83	83	83	83	83	83
	2	2	2		2	2	2	2	2	2	2	2
Heavy Vehicles, % Mvmt Flow	30	388	35	7	248	135	20	0	4	99	0	17
IVIVIIIL FIUW	30	388	35	1	248	135	20	U	4	99	U	17
	lajor1			Major2			Minor1			/linor2		
Conflicting Flow All	383	0	0	423	0	0	604	863	212	584	813	192
Stage 1	-	-		-	-		466	466	-	330	330	
Stage 2	-	-	-	-	-	-	138	397	-	254	483	-
Critical Hdwy	4.14	-	-	4.14	-		7.54	6.54	6.94	7.54	6.54	6.94
Critical Hdwy Stg 1	-	-	-	-	-	-	6.54	5.54	-	6.54	5.54	-
Critical Hdwy Stg 2	-	-		-	-		6.54	5.54	-	6.54	5.54	
Follow-up Hdwy	2.22	-	-	2.22	-	-	3.52	4.02	3.32	3.52	4.02	3.32
Pot Cap-1 Maneuver	1172	-		1133	-		382	291	793	395	311	817
Stage 1	-	-	-	-	-	-	546	561	-	657	644	-
Stage 2	-	-		-	-		851	602	-	728	551	
Platoon blocked, %		-	-		-	-						
	1172			1133	-		365	282	793	384	301	817
Mov Cap-2 Maneuver	-	-	-	-	-	-	365	282	-	384	301	-
Stage 1	-	-	-	-	-	-	532	546	-	640	640	-
Stage 2	-	-	-	-	-	-	828	598	-	706	537	-
Approach	EB			WB	_		NB			SB	_	
HCM Control Delay, s	0.5			0.2			14.7			17		
HCM LOS							В			C		
110111 2000												
Minor Lane/Major Mvmt		NBLn1	EBL	EBT	EBR	WBL	WBT	WBR	CRI n1			
Capacity (veh/h)		397	1172	EDI	LDIK	1133	WDI	WDIX.	416			
								-	0.278			
HCM Cantrol Doloy (s)		0.061	0.026	-	-	0.006	-	-				
HCM Long LOS		14.7	8.2	-	-	8.2	-	-	17			
HCM Lane LOS		В	Α	-	-	A	-	-	C			
HCM 95th %tile Q(veh)		0.2	0.1	-	-	0	-	-	1.1			

Intersection	
Intersection Delay, s/veh	9.7
Intersection LOS	Α

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	7	↑ ↑		7	† 1>			4			44	
Traffic Vol, veh/h	14	153	7	21	96	137	19	2	90	122	5	10
Future Vol, veh/h	14	153	7	21	96	137	19	2	90	122	5	10
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles, %	2	2	2	2	2	2	2	2	2	2	2	2
Mvmt Flow	15	166	8	23	104	149	21	2	98	133	5	11
Number of Lanes	1	2	0	1	2	0	0	1	0	0	1	0
Approach	EB			WB			NB			SB		
Opposing Approach	WB			EB			SB			NB		
Opposing Lanes	3			3			1			1		
Conflicting Approach Left	SB			NB			EB			WB		
Conflicting Lanes Left	1			1			3			3		
Conflicting Approach Right	NB			SB			WB			EB		
Conflicting Lanes Right	1			1			3			3		
HCM Control Delay	9.4			9.3			9.4			11.1		
HCM LOS	Α			Α			Α			В		

Lane	NBLn1	EBLn1	EBLn2	EBLn3	WBLn1	WBLn2	WBLn3	SBLn1	
Vol Left, %	17%	100%	0%	0%	100%	0%	0%	89%	
Vol Thru, %	2%	0%	100%	88%	0%	100%	19%	4%	
Vol Right, %	81%	0%	0%	12%	0%	0%	81%	7%	
Sign Control	Stop								
Traffic Vol by Lane	111	14	102	58	21	64	169	137	
LT Vol	19	14	0	0	21	0	0	122	
Through Vol	2	0	102	51	0	64	32	5	
RT Vol	90	0	0	7	0	0	137	10	
Lane Flow Rate	121	15	111	63	23	70	184	149	
Geometry Grp	7	7	7	7	7	7	7	7	
Degree of Util (X)	0.182	0.026	0.176	0.099	0.039	0.109	0.258	0.258	
Departure Headway (Hd)	5.433	6.231	5.724	5.638	6.145	5.639	5.064	6.234	
Convergence, Y/N	Yes								
Cap	661	577	630	639	586	639	713	578	
Service Time	3.158	3.938	3.431	3.345	3.849	3.343	2.768	3.958	
HCM Lane V/C Ratio	0.183	0.026	0.176	0.099	0.039	0.11	0.258	0.258	
HCM Control Delay	9.4	9.1	9.7	9	9.1	9	9.5	11.1	
HCM Lane LOS	Α	Α	Α	Α	Α	Α	Α	В	
HCM 95th-tile Q	0.7	0.1	0.6	0.3	0.1	0.4	1	1	

Intersection												
Intersection Delay, s/veh	9.3											
Intersection LOS	Α											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBF
Lane Configurations		4						^	7	7	^	
Traffic Vol, veh/h	39	7	5	0	0	0	0	8	46	191	68	(
Future Vol, veh/h	39	7	5	0	0	0	0	8	46	191	68	(
Peak Hour Factor	0.82	0.82	0.82	0.82	0.82	0.82	0.82	0.82	0.82	0.82	0.82	0.82
Heavy Vehicles, %	2	2	2	2	2	2	2	2	2	2	2	2
Mvmt Flow	48	9	6	0	0	0	0	10	56	233	83	(
Number of Lanes	0	1	0	0	0	0	0	1	1	1	2	(
Approach	EB							NB		SB		
Opposing Approach								SB		NB		
Opposing Lanes	0							3		2		
Conflicting Approach Left	SB							EB				
Conflicting Lanes Left	3							1		0		
Conflicting Approach Right	NB									EB		
Conflicting Lanes Right	2							0		1		
HCM Control Delay	9.1							7.8		9.7		
HCM LOS	Α							Α		Α		
Lane		NBLn1	NBLn2	EBLn1	SBLn1	SBLn2	SBLn3					
Vol Left, %		0%	0%	76%	100%	0%	0%					
Vol Thru, %		100%	0%	14%	0%	100%	100%					
Vol Right, %		0%	100%	10%	0%	0%	0%					
Sign Control		Stop	Stop	Stop	Stop	Stop	Stop					
Traffic Vol by Lane		8	46	51	191	34	34					
LT Vol		0	0	39	191	0	0					
Through Vol		8	0	7	0	34	34					
RT Vol		0	46	5	0	0	0					
Lane Flow Rate		10	56	62	233	41	41					
Geometry Grp		8	8	7	7	7	7					
Degree of Util (X)		0.015	0.072	0.099	0.342	0.055	0.035					
Departure Headway (Hd)		5.354	4.651	5.729	5.286	4.784	3.046					
Convergence, Y/N		Yes	Yes	Yes	Yes	Yes	Yes					
Con		470	771	427	405	752	1102					

685 3.078 2.375 3.452 2.986 2.484 0.746

1.5

7.8 5.9

0.2

0.015 0.073 0.099 0.34 0.054 0.035

9.1 10.7

0.3

771 627

0.2

8.2 7.7

Cap Service Time

HCM Lane V/C Ratio

HCM Control Delay HCM Lane LOS HCM 95th-tile Q

Intersection												
Intersection Delay, s/veh	10.7											
Intersection LOS	В											
	- FD:		500	11101		11/00		NET	NDD	0.01		000
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4		7	+			4	
Traffic Vol, veh/h	2	0	3	87	3	128	8	42	0	0	168	190
Future Vol, veh/h	2	0	3	87	3	128	8	42	0	0	168	190
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
leavy Vehicles, %	2	2	2	2	2	2	2	2	2	2	2	2
/Ivmt Flow	2	0	3	97	3	142	9	47	0	0	187	211
Number of Lanes	0	1	0	0	1	0	1	1	0	0	1	0
Approach	EB			WB			NB				SB	
Opposing Approach	WB			EB			SB				NB	
Opposing Lanes	1			1			1				2	
Conflicting Approach Left	SB			NB			EB				WB	
Conflicting Lanes Left	1			2			1				1	
Conflicting Approach Right	NB			SB			WB				EB	
Conflicting Lanes Right	2			1			1				1	
HCM Control Delay	8.1			9.9			8.7				11.5	
HCM LOS	Α			Α			Α				В	
Lane		NBLn1	NBLn2	EBLn1	WBLn1	SBLn1						
/ol Left, %		100%	0%	40%	40%	0%						
/ol Thru, %		0%	100%	0%	1%	47%						
ol Right, %		0%	0%	60%	59%	53%						
ign Control		Stop	Stop	Stop	Stop	Stop						
raffic Vol by Lane		8	42	5	218	358						
.T Vol		8	0	2	87	0						
hrough Vol		0	42	0	3	168						
RT Vol		0	0	3	128	190						
ane Flow Rate		9	47	6	242	398						
		7	7	2	2	5						
Degree of Util (X)		0.015	0.071	0.008	0.316	0.485						
Degree of Util (X) Departure Headway (Hd)		0.015 5.993	0.071 5.488	0.008 5.013	0.316 4.696	0.485 4.392						
Degree of Util (X) Departure Headway (Hd) Convergence, Y/N		0.015 5.993 Yes	0.071 5.488 Yes	0.008 5.013 Yes	0.316 4.696 Yes	0.485 4.392 Yes						
Degree of Util (X) Departure Headway (Hd) Convergence, Y/N Cap		0.015 5.993 Yes 595	0.071 5.488 Yes 650	0.008 5.013 Yes 708	0.316 4.696 Yes 763	0.485 4.392 Yes 819						
Degree of Util (X) Departure Headway (Hd) Convergence, Y/N Cap Service Time		0.015 5.993 Yes 595 3.754	0.071 5.488 Yes 650 3.248	0.008 5.013 Yes 708 3.085	0.316 4.696 Yes 763 2.745	0.485 4.392 Yes 819 2.434						
Degree of Util (X) Departure Headway (Hd) Convergence, Y/N Cap Service Time HCM Lane V/C Ratio		0.015 5.993 Yes 595 3.754 0.015	0.071 5.488 Yes 650 3.248 0.072	0.008 5.013 Yes 708 3.085 0.008	0.316 4.696 Yes 763 2.745 0.317	0.485 4.392 Yes 819 2.434 0.486						
Degree of Util (X) Departure Headway (Hd) Convergence, Y/N Cap Service Time HCM Lane V/C Ratio HCM Control Delay		0.015 5.993 Yes 595 3.754 0.015 8.8	0.071 5.488 Yes 650 3.248 0.072 8.7	0.008 5.013 Yes 708 3.085 0.008 8.1	0.316 4.696 Yes 763 2.745 0.317 9.9	0.485 4.392 Yes 819 2.434 0.486 11.5						
egree of Util (X) eparture Headway (Hd) onvergence, Y/N ap ervice Time CM Lane V/C Ratio CM Control Delay CM Lane LOS		0.015 5.993 Yes 595 3.754 0.015 8.8 A	0.071 5.488 Yes 650 3.248 0.072 8.7	0.008 5.013 Yes 708 3.085 0.008 8.1 A	0.316 4.696 Yes 763 2.745 0.317 9.9 A	0.485 4.392 Yes 819 2.434 0.486 11.5 B						
Geometry Grp Degree of Util (X) Departure Headway (Hd) Convergence, Y/N Cap Service Time HCM Lane V/C Ratio HCM Control Delay HCM Lane LOS HCM 95th-tile Q		0.015 5.993 Yes 595 3.754 0.015 8.8	0.071 5.488 Yes 650 3.248 0.072 8.7	0.008 5.013 Yes 708 3.085 0.008 8.1	0.316 4.696 Yes 763 2.745 0.317 9.9	0.485 4.392 Yes 819 2.434 0.486 11.5						

Intersection						
Int Delay, s/veh	1.7					
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	W		1100	4	13	00.1
Traffic Vol, veh/h	17	31	53	119	326	29
Future Vol, veh/h	17	31	53	119	326	29
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	- Jiop	None	-	None	-	None
Storage Length	0	-		-		-
Veh in Median Storage	-			0	0	
Grade, %	0			0	0	
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	2	92	2	92	2
Mymt Flow	18	34	58	129	354	32
IVIVIIII FIOW	18	34	58	129	354	32
Major/Minor	Minor2		Major1	N	/lajor2	
Conflicting Flow All	615	370	386	0	-	0
Stage 1	370	-			-	
Stage 2	245					
Critical Hdwy	6.42	6.22	4.12			
Critical Hdwy Stg 1	5.42	0.22				
Critical Hdwy Stg 2	5.42					
Follow-up Hdwy	3.518	3.318	2.218			
Pot Cap-1 Maneuver	455	676	1172			
Stage 1	699	070	1172			
Stage 2	796				- :	
Platoon blocked. %	770					
Mov Cap-1 Maneuver	431	676	1172			-
Mov Cap-1 Maneuver	431	0/0	11/2			
	662		-		-	-
Stage 1		-	-		-	
Stage 2	796	-	-	-	-	-
Approach	EB		NB		SB	
HCM Control Delay, s	12		2.5		0	
HCM LOS	В		2.0		0	
TIOW EOS	U					
Minor Lane/Major Mvn	nt	NBL	NBT	EBLn1	SBT	SBR
Capacity (veh/h)		1172	-	563	-	-
HCM Lane V/C Ratio		0.049	-	0.093	-	-
HCM Control Delay (s)		8.2	0	12	-	
HCM Lane LOS		Α	Α	В	-	-
HCM 95th %tile Q(veh)	0.2		0.3	-	
	,					

HIGHWAY CAPACITY MANUAL LEVEL OF SERVICE DESCRIPTIONS

Level of Service Criteria for Signalized Intersections

Level-of-Service (LOS)	Average Control Delay (seconds per vehicle)	Description
А	≤ 10.0	Very low vehicle delays, free flow, signal progression extremely favorable, most vehicles arrive during given signal phase.
В	10.1 - 20.0	Good signal progression, more vehicles stop and experience higher delays than for LOS A.
С	20.1 - 35.0	Stable flow, fair signal progression, significant number of vehicles stop at signals.
D	35.1 - 55.0	Congestion noticeable, longer delays and unfavorable signal progression, many vehicles stop at signals.
E	55.1 - 80.0	Limit of acceptable delay, unstable flow, poor signal progression, traffic near roadway capacity, frequent cycle failures.
F	> 80.0	Unacceptable delays, extremely unstable flow and congestion, traffic exceeds roadway capacity, stop-and-go conditions.

SOURCE: Highway Capacity Manual, HCM 2010, Transportation Research Board, 2010.

Level of Service Criteria for Unsignalized Intersections

Level-of-Service (LOS)	Average Control Delay (seconds per vehicle)	Description
А	≤ 10.0	No delays at intersections with continuous flow of traffic. Uncongested operations: high frequency of long gaps available for all left and right turning traffic. No observable queues.
В	10.1 - 15.0	No delays at intersections with continuous flow of traffic. Uncongested operations: high frequency of long gaps available for all left and right turning traffic. No observable queues.
С	15.1 - 25.0	Moderate delays at intersections with satisfactory to good traffic flow. Light congestion; infrequent backups on critical approaches.
D	25.1 - 35.0	Increased probability of delays along every approach. Significant congestion on critical approaches, but intersection functional. No standing long lines formed.
E	35.1 - 50.0	Heavy traffic flow condition. Heavy delays probable. No available gaps for cross-street traffic or main street turning traffic. Limit of stable flow.
F	> 50.0	Unstable traffic flow. Heavy congestion. Traffic moves in forced flow condition. Average delays greater than one minute highly probable. Total breakdown.

SOURCE: Highway Capacity Manual, HCM 2010, Transportation Research Board, 2010.

ORDINANCE NO.

AN ORDINANCE AMENDING APPENDIX A "UNIFIED DEVELOPMENT ORDINANCE," ARTICLE 4 "ZONING DISTRICTS," SECTION 4.2, "OFFICIAL ZONING MAP" OF THE CODE OF ORDINANCES OF THE CITY OF COLLEGE STATION, TEXAS, BY CHANGING THE ZONING DISTRICT BOUNDARIES FROM R RURAL TO MF MULTI-FAMILY FOR APPROXIMATLEY 11.1 ACRES GENERALLY LOCATED IN THE CITY ON THE WEST SIDE OF TURKEY CREEK ROAD, APPROXIMATELY 500-FEET SOUTH OF HSC PARKWAY; PROVIDING A SEVERABILITY CLAUSE; DECLARING A PENALTY; AND PROVIDING AN EFFECTIVE DATE.

BE IT ORDAINED BY THE CITY COUNCIL OF THE CITY OF COLLEGE STATION, TEXAS:

- **PART 1:** That Appendix A "Unified Development Ordinance," Article 4 "Zoning Districts," Section 4.2 "Official Zoning Map" of the Code of Ordinances of the City of College Station, Texas, be amended as set out in **Exhibit "A," Exhibit "B"** and **Exhibit "C"** attached hereto and made a part of this Ordinance for all purposes.
- **PART 2:** If any provision of this Ordinance or its application to any person or circumstances is held invalid or unconstitutional, the invalidity or unconstitutionality does not affect other provisions or application of this Ordinance or the Code of Ordinances of the City of College Station, Texas, that can be given effect without the invalid or unconstitutional provision or application, and to this end the provisions of this Ordinance are severable.
- **PART 3:** That any person, corporation, organization, government, governmental subdivision or agency, business trust, estate, trust, partnership, association and any other legal entity violating any of the provisions of this Ordinance shall be deemed guilty of a misdemeanor, and upon conviction thereof shall be punishable by a fine of not less than twenty five dollars (\$25.00) and not more than five hundred dollars (\$500.00) or more than two thousand dollars (\$2,000) for a violation of fire safety, zoning, or public health and sanitation ordinances, other than the dumping of refuse. Each day such violation shall continue or be permitted to continue, shall be deemed a separate offense.
- **PART 4:** This Ordinance is a penal ordinance and becomes effective ten (10) days after its date of passage by the City Council, as provided by City of College Station Charter Section 35.

PASSED, ADOPTED, and APPROV	ED this 23rd day of August, 2018.
ATTEST:	APPROVED:
City Secretary	Mayor
APPROVED:	
City Attorney	

ORDINANCE NO.

Page **3** of **6**

Exhibit A

That Appendix A "Unified Development Ordinance," Article 4 "Zoning Districts," Section 4.2, "Official Zoning Map" of the Code of Ordinances of the City of College Station, Texas, is hereby amended to read as follows:

The following property is rezoned from R Rural to MF Multi-Family:

METES AND BOUNDS DESCRIPTION OF A 11.14 ACRE TRACT J. H. JONES SURVEY, A-26 COLLEGE STATION, BRAZOS COUNTY, TEXAS

METES AND BOUNDS DESCRIPTION OF ALL THAT CERTAIN TRACT OR PARCEL OF LAND LYING AND BEING SITUATED IN THE J. H. JONES SURVEY, ABSTRACT NO. 26, COLLEGE STATION, BRAZOS COUNTY, TEXAS. SAID TRACT BEING THE REMAINDER OF A CALLED 11.6 ACRE TRACT AS DESCRIBED BY A DEED TO LOVELESS ACRES, L.C. RECORDED IN VOLUME 7004, PAGE 104 OF THE OFFICIAL PUBLIC RECORDS OF BRAZOS COUNTY, TEXAS.

SAID TRACT BEING MORE PARTICULARLY DESCRIBED BY METES AND BOUNDS AS FOLLOWS:

BEGINNING AT A 1/2 INCH IRON ROD FOUND IN A TREE ROOT ON THE SOUTHWEST LINE OF TURKEY CREEK ROAD (FM 2513 - 100' R.O.W.) MARKING THE NORTH CORNER OF SAID REMAINDER OF 11.6 ACRE TRACT AND AN EAST CORNER OF THE REMAINDER OF A CALLED 417.85 ACRE TRACT DESCRIBED AS EXHIBIT "A" BY A DEED TO BRYAN COMMERCE AND DEVELOPMENT, INC. RECORDED IN VOLUME 4023, PAGE 91 OF THE OFFICIAL PUBLIC RECORDS OF BRAZOS COUNTY, TEXAS;

THENCE: \$ 32° 14' 41" E ALONG THE SOUTHWEST LINE OF TURKEY CREEK ROAD FOR A DISTANCE OF 722.89 FEET TO A POINT WITHIN AN EXISTING ELM TREE MARKING THE NORTH CORNER OF A CALLED 7.02 ACRE TRACT AS DESCRIBED BY A DEED TO CASHION FAMILY LIMITED PARTNERSHIP RECORDED IN VOLUME 11415, PAGE 138 OF THE OFFICIAL PUBLIC RECORDS OF BRAZOS COUNTY, TEXAS, AND THE EAST CORNER OF THIS HEREIN DESCRIBED TRACT;

THENCE: S 53° 19' 32" W ALONG THE COMMON LINE OF SAID REMAINDER OF 11.6 ACRE TRACT AND SAID 7.02 ACRE TRACT FOR A DISTANCE OF 658.46 FEET TO A 1/2 INCH IRON ROD FOUND ON THE NORTHEAST LINE OF SAID REMAINDER OF 417.85 ACRE TRACT MARKING THE COMMON CORNER OF SAID 7.02 ACRE TRACT AND SAID REMAINDER OF 11.6 ACRE TRACT;

THENCE: N 35° 49' 44" W ALONG THE COMMON LINE OF SAID REMAINDER OF 11.6 ACRE TRACT AND SAID REMAINDER OF 417.85 ACRE TRACT FOR A DISTANCE OF 704.85 FEET TO A CONCRETE MONUMENT FOUND MARKING THE WEST CORNER OF THIS HEREIN DESCRIBED TRACT;

THENCE: N 52° 01' 36" E CONTINUING ALONG THE COMMON LINE OF SAID REMAINDER OF 11.6 ACRE TRACT AND SAID REMAINDER OF 417.85 ACRE TRACT FOR A DISTANCE OF 704.08 FEET TO THE **POINT OF BEGINNING** CONTAINING 11.14 ACRES OF LAND, MORE OF LESS, AS SURVEYED ON THE GROUND. BEARING SYSTEM SHOWN HEREIN IS BASED ON NAD 83 TEXAS STATE PLANE COORDINATES AS ESTABLISHED FROM GPS OBSERVATION. SEE PLAT PREPARED DECEMBER 2016 FOR MORE DESCRIPTIVE INFORMATION.

BRAD KERR REGISTERED PROFESSIONAL LAND SURVEYOR No. 4502

//JOBS/16-885/16-885.DOCX



Exhibit B

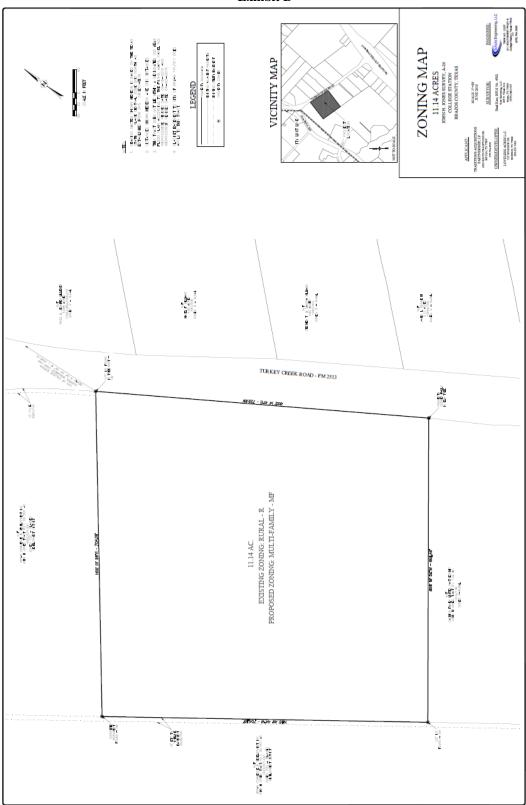


Exhibit C

